

150 & 160 KING STREET EAST, THORNBURY
2706499 ONTARIO INC.
TOWN OF THE BLUE MOUNTAINS

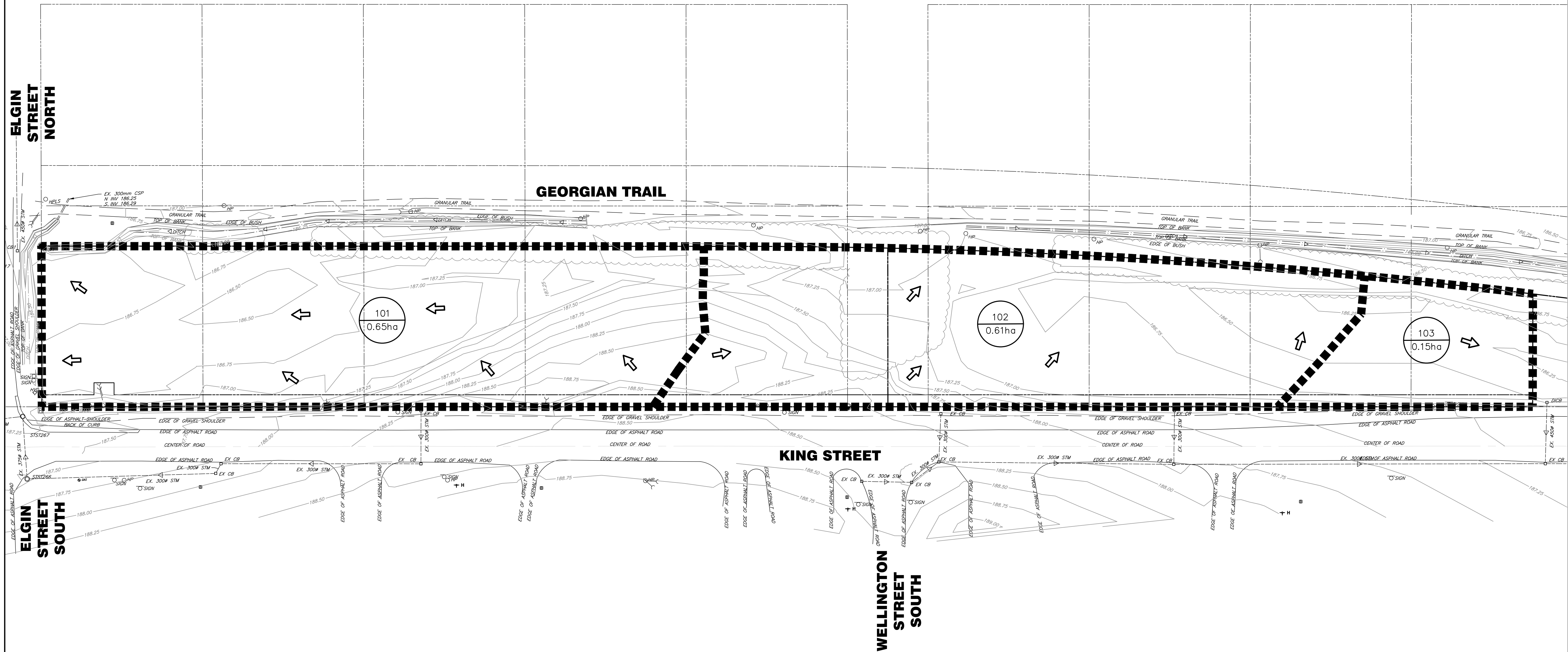
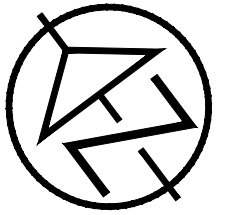
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KEY PLAN

**Contract No. 122030**

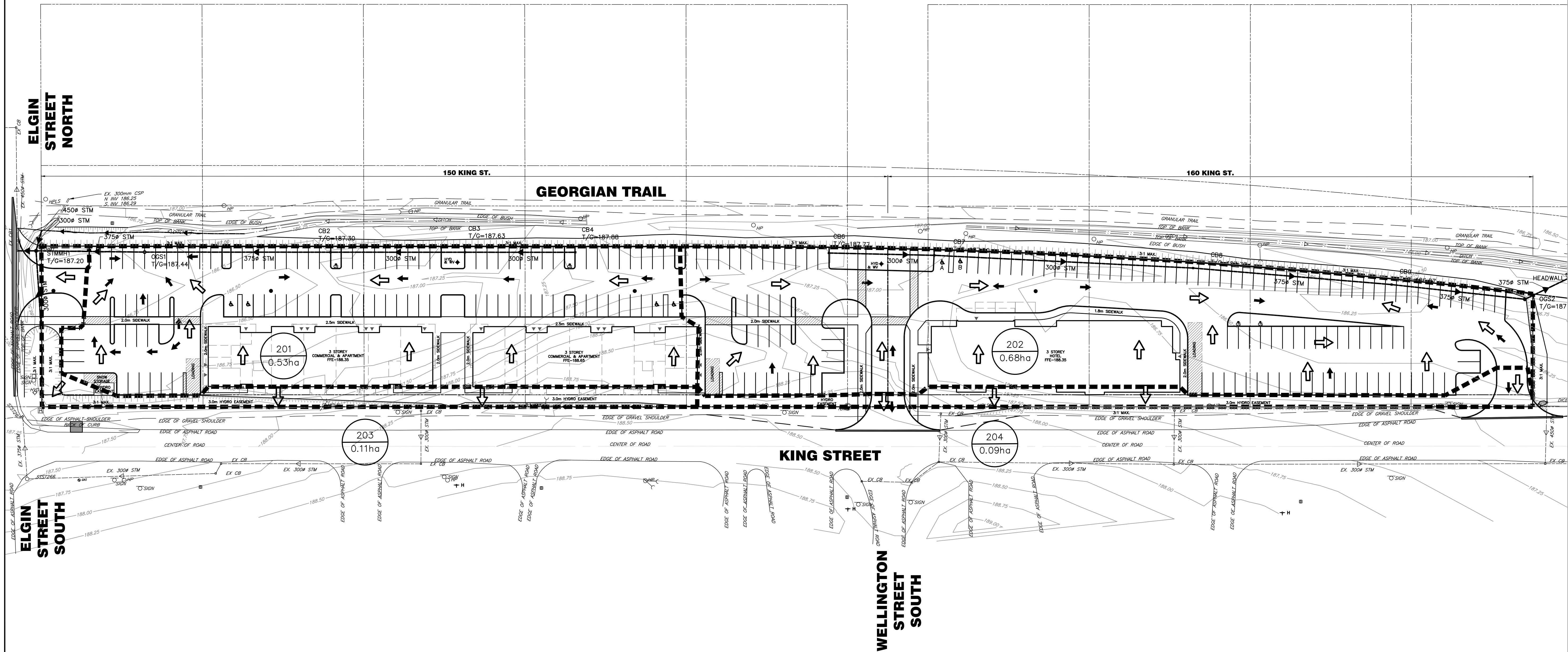
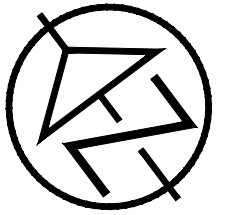
LEGEND

	EXISTING SANITARY MAIN/ SIZE /DIRECTION OF FLOW
	EXISTING STORM SEWER/ SIZE/ DIRECTION OF FLOW
	EXISTING WATERMAIN/ SIZE
----	EXISTING GAS MAIN
----	EXISTING SANITARY SERVICE
----	EXISTING PROPERTY LINE
----	EXISTING EASEMENT LINE
----	EXISTING CENTERLINE
----	EXISTING EDGE OF ASPHALT
----	EXISTING EDGE OF SHOULDER
----	EXISTING DITCH
-x-x-x-x-	EXISTING FENCE LINE
	EXISTING EDGE OF BUSH
	EXISTING TEMPORARY BENCHMARK
	EXISTING BOREHOLE/ NUMBER
=====	EXISTING SANITARY MANHOLE/ NUMBER
=====	EXISTING CULVERT
	EXISTING WATERMAIN PLUG AND THRUST BLOCK
	EXISTING CABLE PEDESTAL
	EXISTING BELL MANHOLE
	EXISTING BELL PEDISTAL
	EXISTING BELL POLE
	EXISTING HYDRO POLE
	EXISTING HYDRO GUY POLE
	EXISTING HYDRO GUY WIRE
	EXISTING GAS MARKER
	EXISTING STANDARD IRON BAR
	EXISTING TRAFFIC SIGN
	EXISTING DECIDUOUS TREE
	PROPOSED SANITARY SEWER/ SIZE/ DIRECTION OF FLOW
	PROPOSED STORM SEWER/ SIZE/ DIRECTION OF FLOW
----	PROPOSED WATERMAIN/SIZE
----	PROPOSED SANITARY SERVICE
----	PROPOSED WATER SERVICE
----	PROPOSED DITCH
----	PROPERTY LINE
----	LOT LINE
----	PROPOSED CENTERLINE
----	PROPOSED EDGE OF ASPHALT
----	PROPOSED EDGE OF SHOULDER
----	PROPOSED PRIVACY FENCE (SEE LANDSCAPE PLANS)
----	PROPOSED CHAINLINK FENCE (SEE LANDSCAPE PLANS)
----	PROPOSED RETAINING WALL
----	PROPOSED RETAINING WALL WITH ACOUSTIC BARRIER
	PROPOSED SANITARY MANHOLE/ NUMBER
	PROPOSED SANITARY CLEANOUT
	PROPOSED DITCH INLET CATCHBASIN
	PROPOSED STORM MANHOLE/ NUMBER
	PROPOSED CATCHBASIN MANHOLE/ NUMBER
	PROPOSED DOUBLE CATCHBASIN MANHOLE/ NUMBER
	PROPOSED CATCHBASIN
	PROPOSED DOUBLE CATCHBASIN
	PROPOSED HEADWALL
=====	PROPOSED CULVERT
	PROPOSED RIPRAP
	PROPOSED HYDRANT & WATER VALVE
	PROPOSED WATER VALVE
	PROPOSED WATER VALVE CHAMBER
	PROPOSED PRESSURE REDUCING WATER VALVE CHAMBER
	PROPOSED AIR RELIEF VALVE CHAMBER
	PROPOSED BLOWOFF
	PROPOSED WATERMAIN PLUG
	PROPOSED PAD MOUNT HYDRO TRANSFORMER
	PROPOSED BELL GRADE LEVEL BOX
	PROPOSED BELL PEDESTAL
	PROPOSED CABLE PEDESTAL
	PROPOSED BOLLARD LUMINAIRE
	PROPOSED LIGHT STANDARD
	PROPOSED CHARGING STATION
	PROPOSED CURB CUT
	PROPOSED TRAFFIC SIGN
	VERTICAL POINT OF INFLECTION
	PROPOSED COMMUNITY MAILBOX
	ROCK CHECK DAM
	STRAW BALE CHECK DAM
	T31.0 (3PR)
	T31.0 (3PR)



LEGEND	
CATCHMENT ID	101
CATCHMENT AREA (ha)	0.5ha
CATCHMENT BOUNDARY	
EXISTING MAJOR OVERLAND FLOW	
EXISTING CONTOUR/ELEVATION	280.0

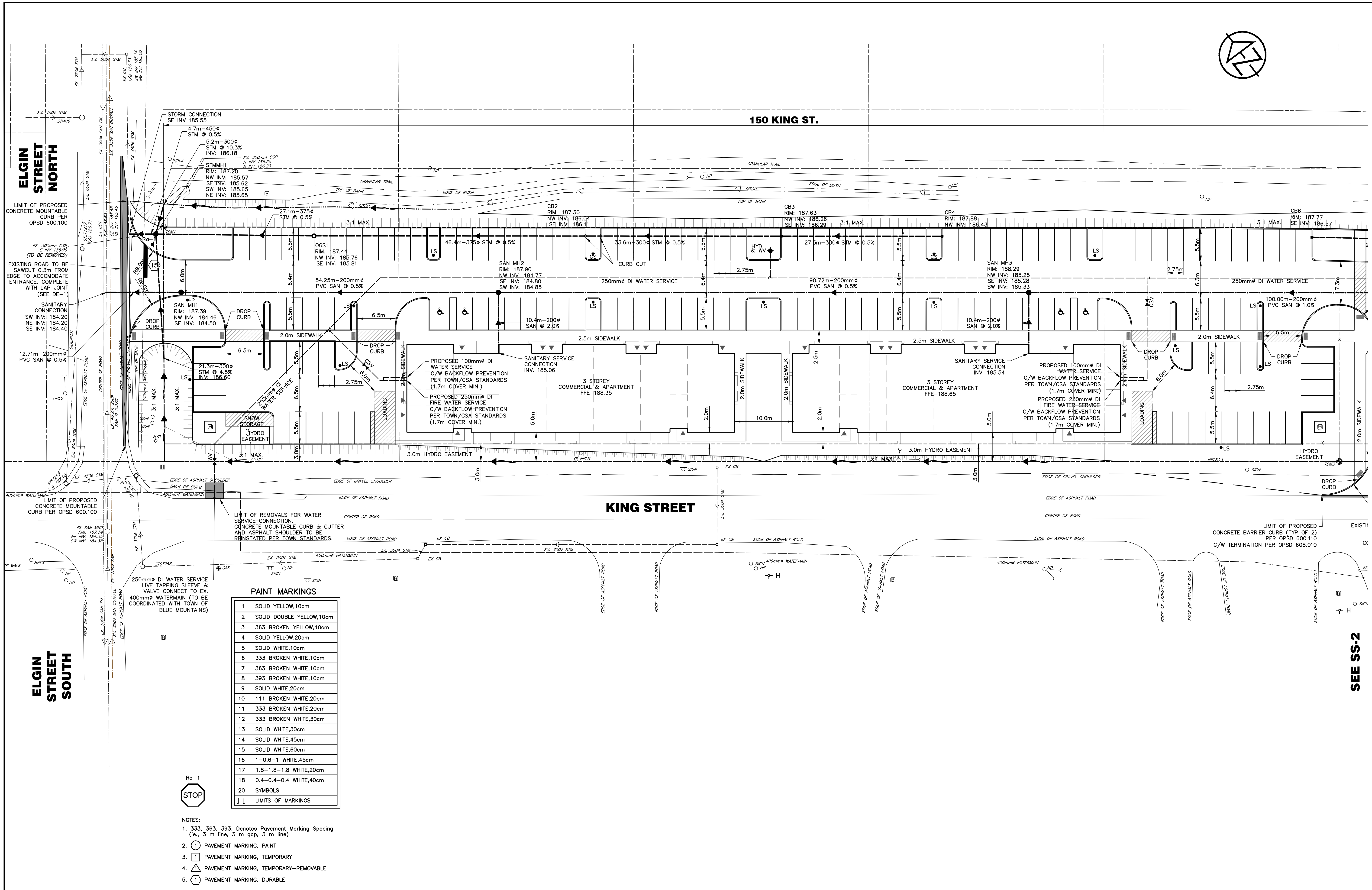
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										PRE-DEVELOPMENT DRAINAGE PLAN		DESIGN: RD	FILE: 122030	DWG:
												DRAWN: RD	DATE: APR 2022	
								CHECK: RS	SCALE: 1:500					



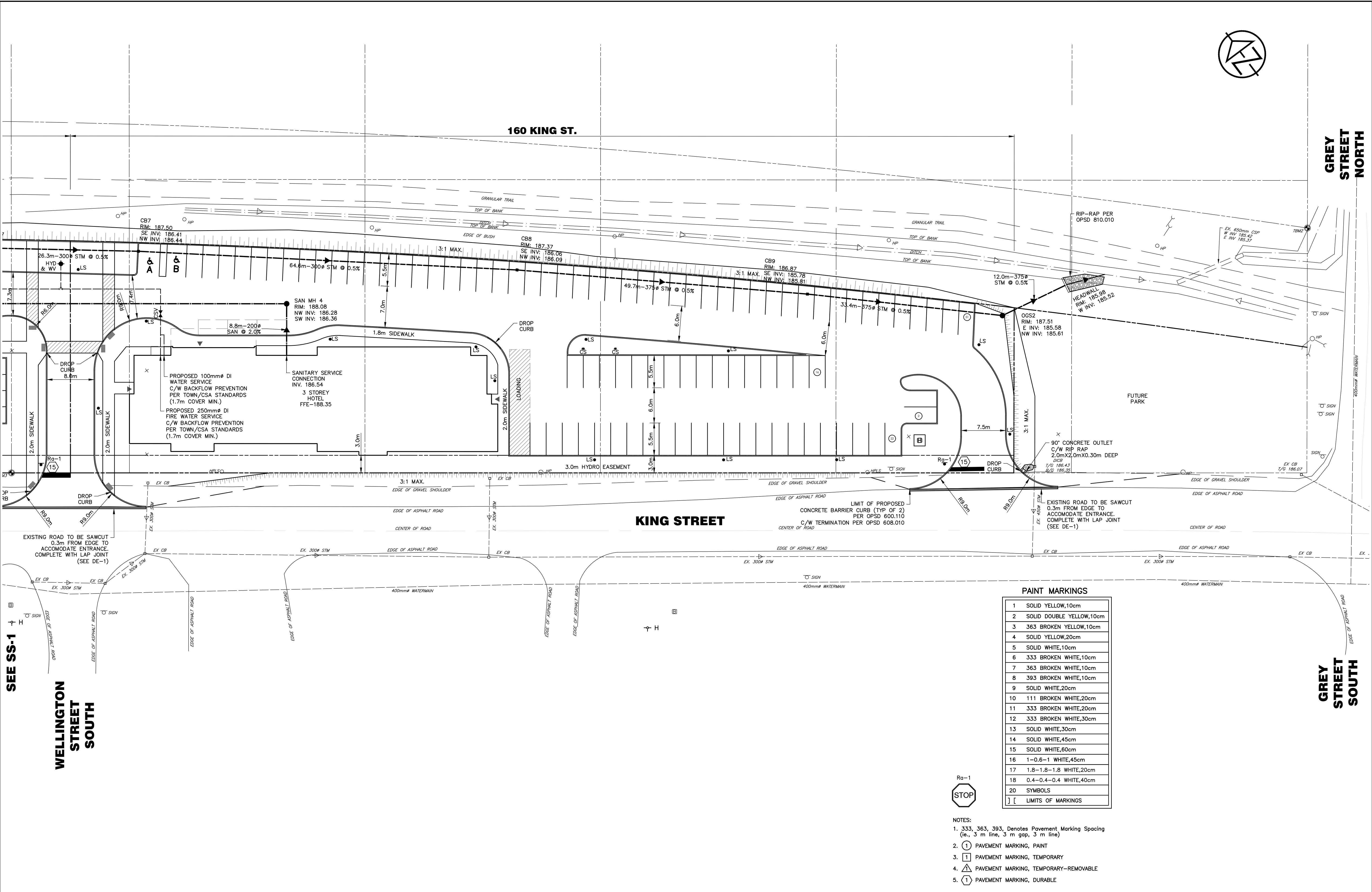
LEGEND

- CATCHMENT ID — 201
- CATCHMENT AREA (ha) — 0.5ha
- CATCHMENT BOUNDARY — — — — —
- PROPOSED MAJOR OVERLAND FLOW —>
- PROPOSED MINOR OVERLAND FLOW —>
- EXISTING CONTOUR/ELEVATION — 187.0

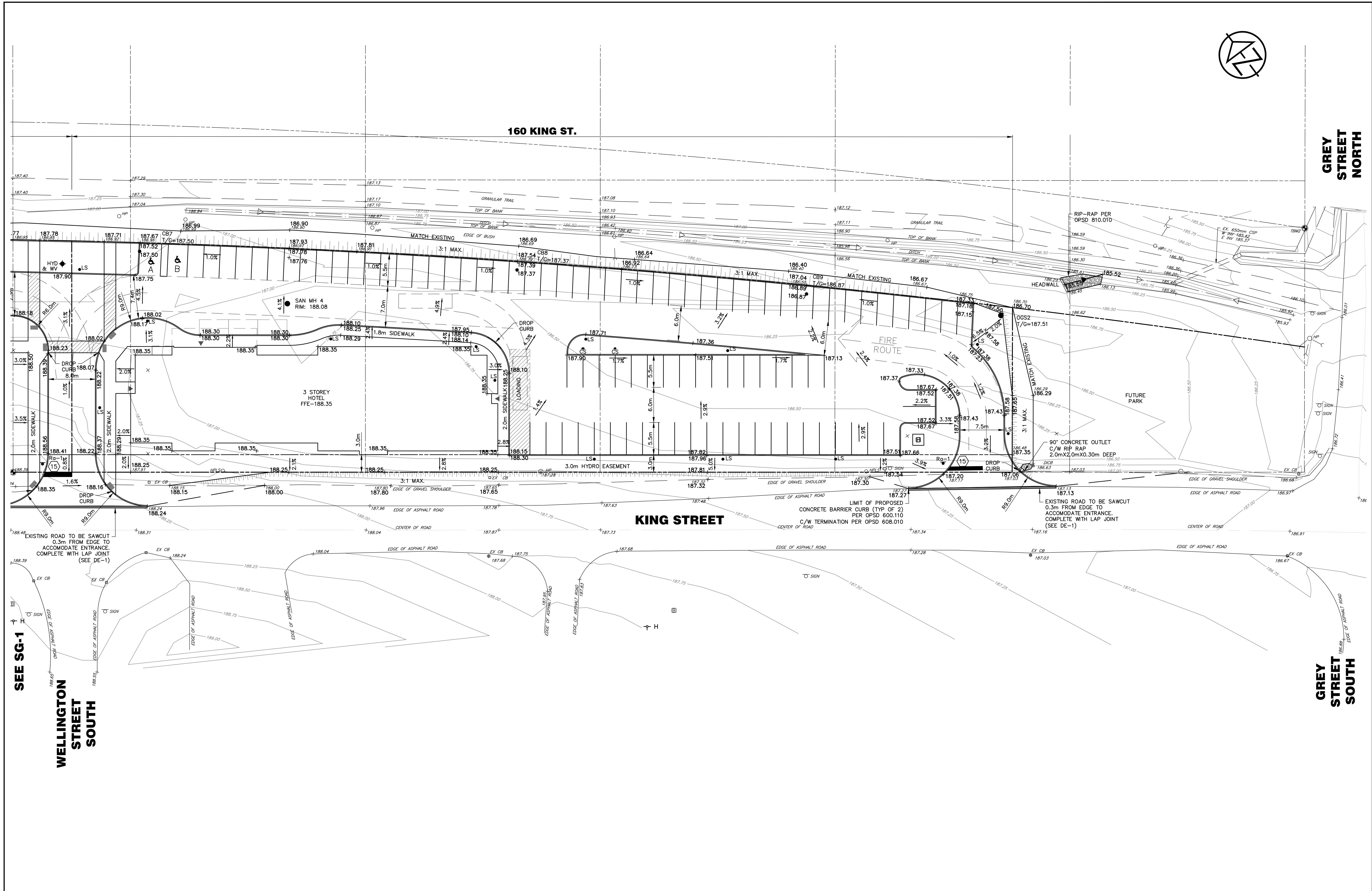
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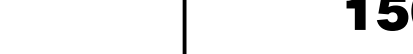


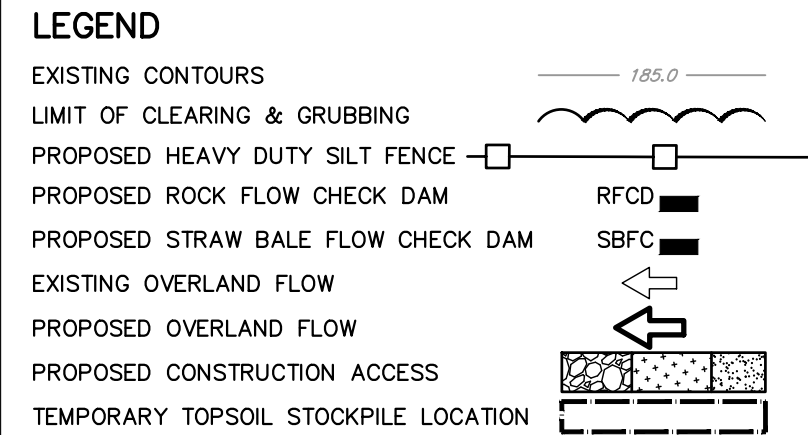
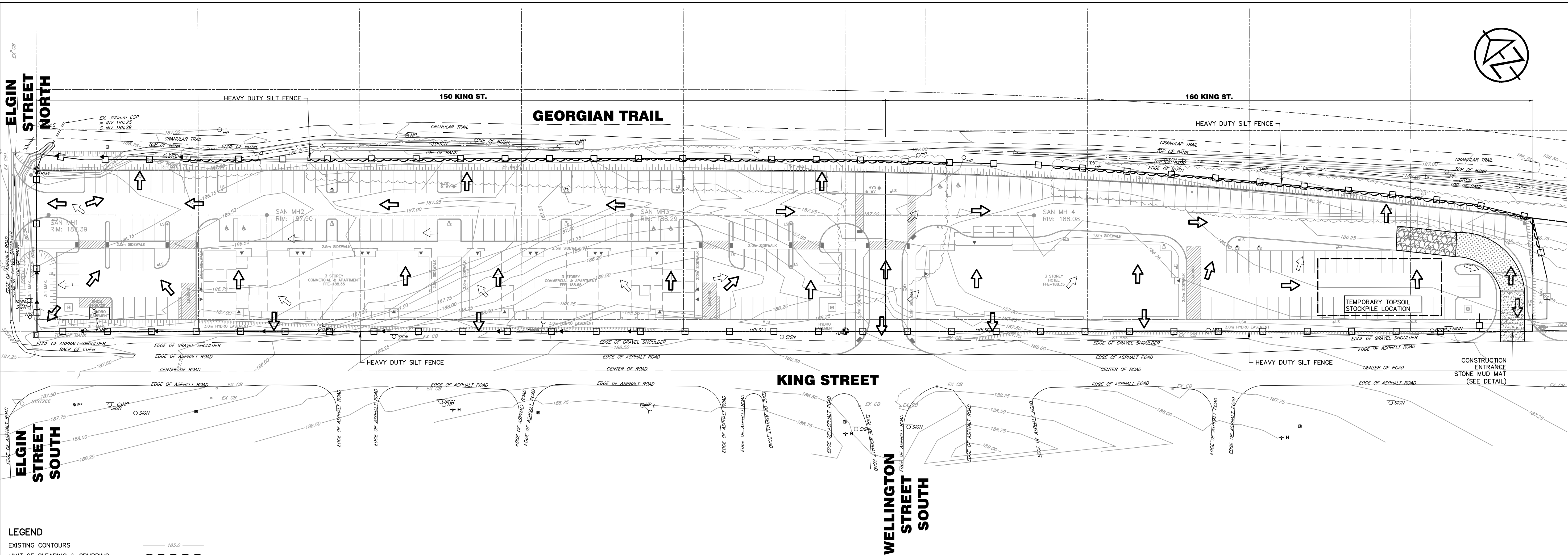
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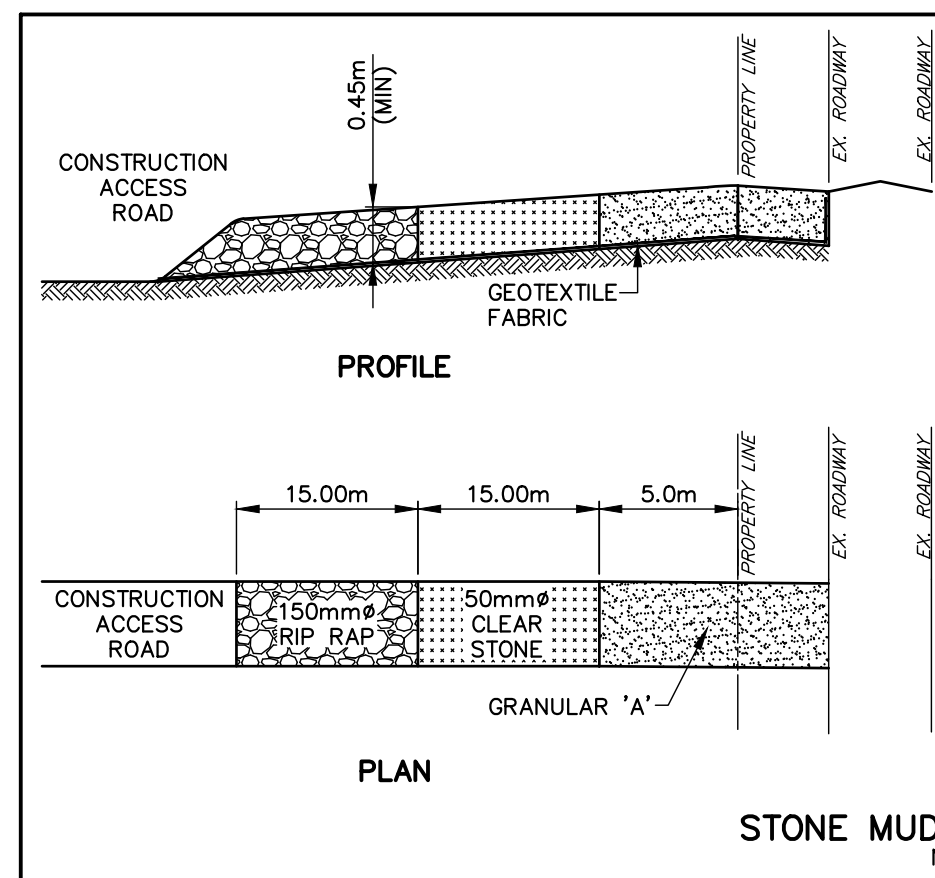
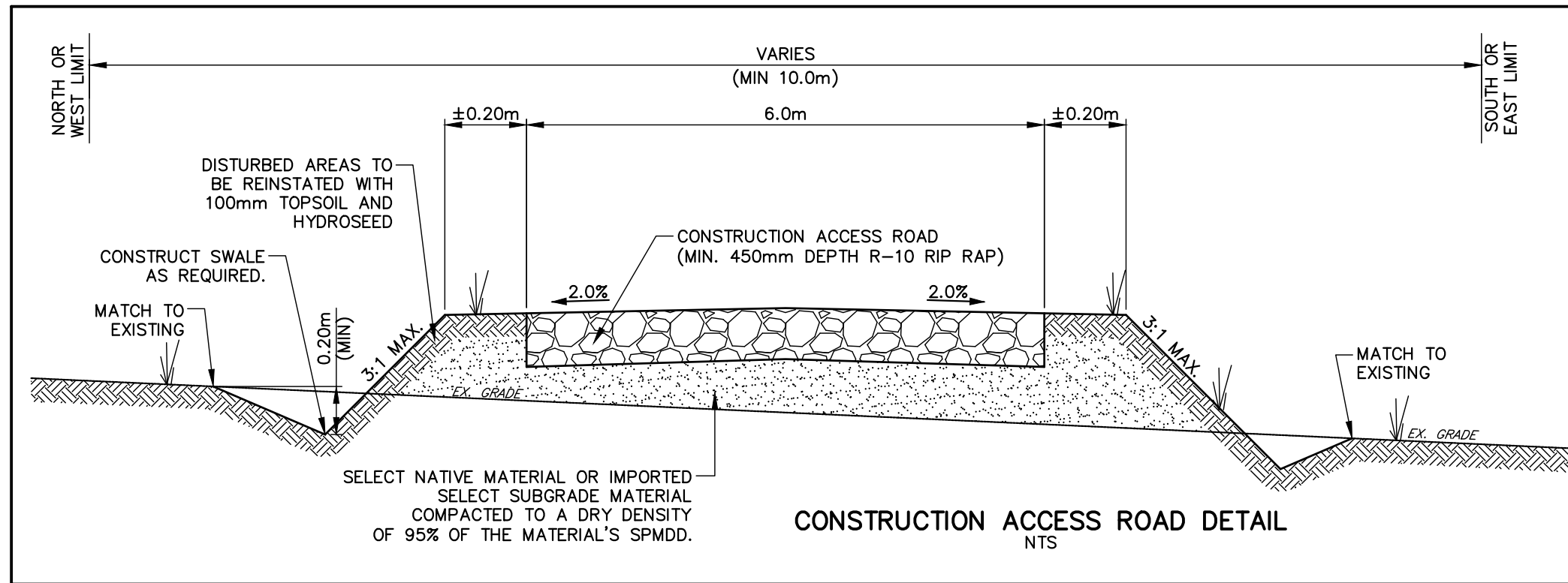
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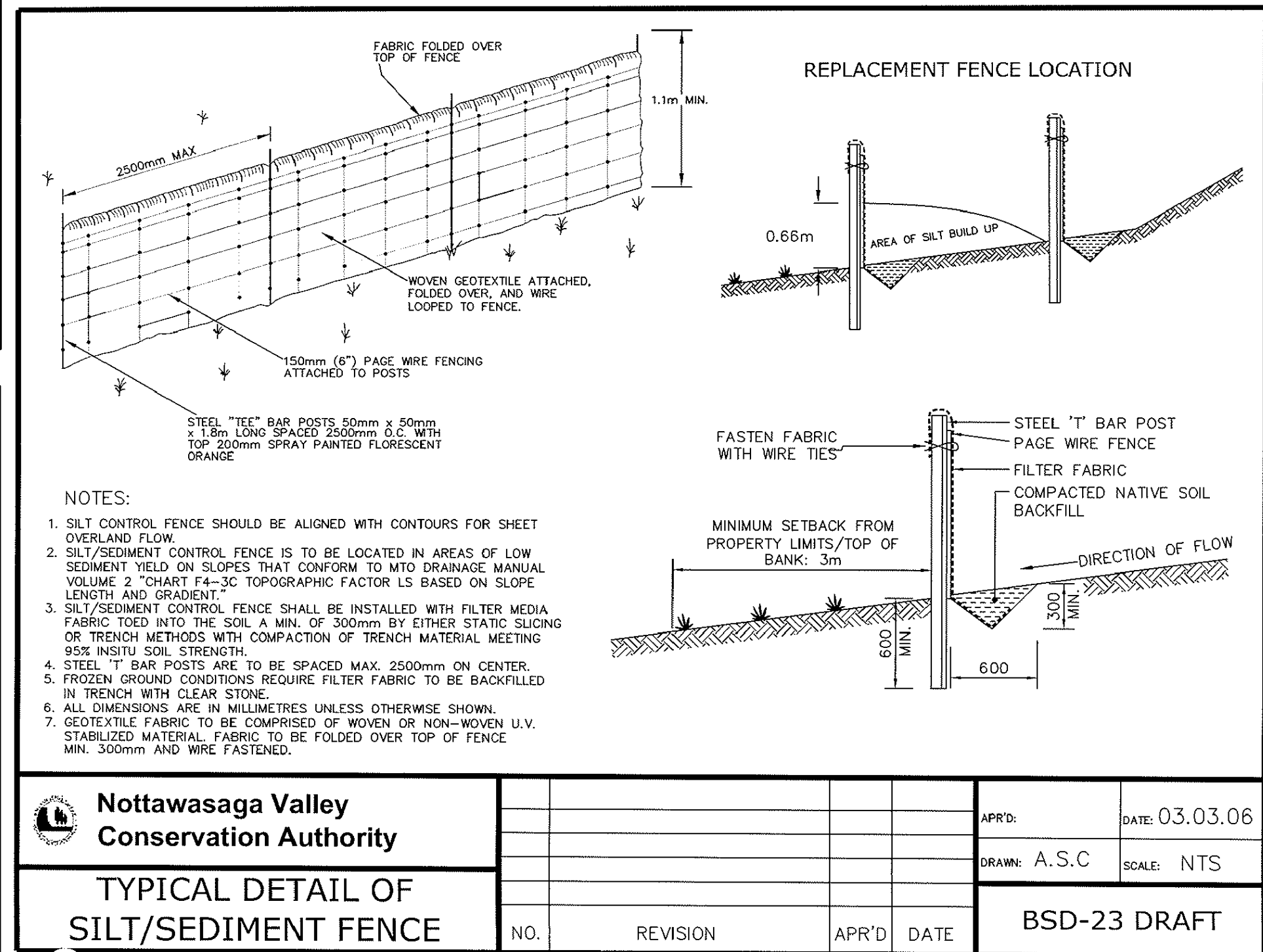


- NOTES**
- ALL SEDIMENT AND EROSION CONTROL MEASURES SHALL BE INSTALLED PRIOR TO THE COMMENCEMENT OF CONSTRUCTION. SEDIMENT AND EROSION CONTROL MEASURES THAT ARE DESIGNED TO CONTROL RUNOFF FROM SPECIFIC AREAS MUST BE INSTALLED PRIOR TO ANY DISTURBANCE OF THAT PART OF THE SITE. THE LOCATION OF ALL SILTATION AND EROSION CONTROL WORKS TO BE REVIEWED ON SITE AND MAY BE REVISED AS DIRECTED BY THE ENGINEER.
 - THE CONTRACTOR MAY CONSIDER ALTERNATIVE SEDIMENT AND EROSION CONTROL MEASURES. SUCH MEASURES MUST BE PRESENTED IN WRITING TO THE ENGINEER FOR APPROVAL OF THE TOWN AND NOTTAWASAGA VALLEY CONSERVATION AUTHORITY.
 - THE CONTRACTOR SHALL HAVE MATERIALS AVAILABLE ON SITE TO REPAIR SEDIMENT AND EROSION CONTROL MEASURES IN THE EVENT OF UNFORESEEN CONDITIONS SUCH AS HIGH WATER, EXTREME RAINFALL EVENTS, ETC.
 - ALL EROSION AND SEDIMENT CONTROL MEASURES MUST BE INSPECTED, CLEANED AND MAINTAINED BY THE CONTRACTOR AFTER EACH STORM EVENT. ALL WORKS WILL BE INSPECTED BY THE ENGINEER BI-WEEKLY AND AFTER EACH MAJOR STORM EVENT.
 - CONSTRUCTION OF ALL SILTATION AND EROSION CONTROL WORK IS TO BE IN ACCORDANCE WITH THE FOLLOWING STEPS:
 - INSTALL NEW STONE MUD MAT AS PER DETAIL.
 - INSTALL SILT FENCE AS PER NOTTAWASAGA VALLEY CONSERVATION AUTHORITY DRAWING BSD-23 DRAFT.
 - INSTALL ROCK FLOW CHECK DAMS AS PER OPSPD 219.211.
 - INSTALL STRAW BALE FLOW CHECKS AS PER OPSPD 219.180.
 - INSTALL TEMPORARY CATCH BASIN SEDIMENT TRAPS ON ALL NEW AND EXISTING CATCH BASINS. SEDIMENT TRAPS TO BE RECTANGULAR CBST BY LAYFIELD OR APPROVED EQUAL. ALL CATCH BASINS TO REMAIN SCREENED UNTIL BASE COURSE ASPHALT IS PLACED AND LOT GRADING IS COMPLETE.
 - ALL CONSTRUCTION VEHICLES TO ACCESS SITE USING THE DESIGNATED CONSTRUCTION ACCESS POINTS.
 - EROSION AND SEDIMENT CONTROL MEASURES TO BE REMOVED BY THE CONTRACTOR ONCE GROUND COVER IS ESTABLISHED AND LANDSCAPING IS COMPLETE AND APPROVED BY THE ENGINEER.
 - STOCKPILE LOCATIONS ARE TO BE APPROVED BY THE ENGINEER.
 - PROVIDE FENCE OR APPROVED EQUAL ACROSS ALL CONSTRUCTION ACCESSES DURING PERIODS OF INACTIVITY.
 - CONSTRUCTION AREAS THAT EXCEED 30 DAYS OF INACTIVITY SHALL BE STABILIZED BY SEEDING IN ACCORDANCE WITH THE NOTTAWASAGA VALLEY CONSERVATION AUTHORITY'S TECHNICAL DESIGN GUIDELINES, STANDARDS AND POLICIES FOR SILTATION AND EROSION CONTROL. CONSTRUCTION CONTROL REQUIREMENTS, NOTES 1, 2 AND 3 AND/OR AS DIRECTED BY THE TOWN. THIS IS TO INCLUDE STOCKPILES OF FILL AND TOPSOIL.



- NOTES:**
- CLEARING AND GRUBBING TO BE COMPLETED AS REQUIRED TO CONSTRUCT THE CONSTRUCTION ACCESS. TEMPORARY SWALES, ETC. IN ACCORDANCE WITH OPSS 201 AND OPSS 180.
 - TOPSOIL TO BE STRIPPED WITHIN THE LIMITS OF THE ACCESS ROAD (MIN. 10.0 m WIDE) AND STOCKPILED IN THE APPROVED STOCKPILE LOCATIONS IN ACCORDANCE WITH OPSS 206 AND OPSS 180.
 - THE CONTRACTOR TO ENSURE POSITIVE DRAINAGE AWAY FROM CONSTRUCTION ACCESS ROAD.
 - TEMPORARY SWALES AND CULVERTS TO GENERALLY BE INSTALLED AS SHOWN ON DWG SC-1. EXACT LOCATIONS TO BE REVIEWED IN THE FIELD AND APPROVED BY ENGINEER PRIOR TO CONSTRUCTION TO ENSURE DRAINAGE IS NOT BLOCKED.
 - DISTURBED AREAS TO BE REINSTATED WITH 100 mm TOPSOIL AND HYDROSEED.

- NOTES**
- STONE SIZE - THE STONE PAD SHALL BE A MIN. 450 mm THICK. USE GRANULAR 'A' FOR THE FIRST 5 m FROM THE PROPERTY LINE, 50 mm Ø CLEAR STONE FOR NEXT 15 m AND 150 mm Ø RIP RAP FOR THE REMAINING 15 m. GRANULAR 'A' BETWEEN PROPERTY LINE AND EDGE OF PAVEMENT.
 - LENGTH - AS REQUIRED BUT NOT LESS THAN 35 m.
 - WIDTH - 7.0 m MIN. BUT NOT LESS THAN THE WIDTH AT POINTS WHERE INGRESS AND EGRESS OCCURS.
 - GEOTEXTILE FABRIC - TERRAFIX 270R OR APPROVED EQUAL TO BE PLACED OVER THE ENTIRE AREA PRIOR TO PLACING STONE.
 - SURFACE WATER - ALL SURFACE WATER FLOWING OR DIRECTED TOWARD CONSTRUCTION ACCESS SHALL BE DIRECTED ACROSS THE ACCESS THROUGH A TEMPORARY CULVERT.
 - MAINTENANCE - THE CONTRACTOR SHALL MAINTAIN THE MUD MAT IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO THE PUBLIC RIGHT-OF-WAY. THIS MAY REQUIRE PERIODIC DRESSING WITH ADDITIONAL STONE AS CONDITIONS DEMAND AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ONTO THE PUBLIC RIGHT-OF-WAY MUST BE REMOVED IMMEDIATELY BY THE CONTRACTOR.
 - WASHING - WHEELS SHALL BE CLEANED TO REMOVE SEDIMENT PRIOR TO ACCESS ONTO THE PUBLIC RIGHT-OF-WAY. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH STONE AND WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE.
 - INSPECTION & MAINTENANCE - REQUIRED MAINTENANCE AFTER EACH RAIN EVENT SHALL BE PROVIDED BY THE CONTRACTOR.
 - REMOVAL - STONE MUD MAT TO REMAIN IN PLACE UNTIL COMMENCEMENT OF ROAD CONSTRUCTION



DISCLAIMER AND COPYRIGHT

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BENCHMARKS

TBM1 - ELEVATION 186.52
ELEVATIONS SHOWN HEREON ARE GEODETIC AND REFER TO THE TOP OF IRON BAR, NORTH WEST CORNER OF LOT.

TBM2 - ELEVATION 185.37
ELEVATIONS SHOWN HEREON ARE GEODETIC AND REFER TO THE TOP OF IRON BAR, NORTH EAST CORNER OF LOT.

TBM3 - ELEVATION 188.63
ELEVATIONS SHOWN HEREON ARE GEODETIC AND REFER TO THE TOP OF IRON BAR, SOUTH WEST CORNER OF WELLINGTON STREET ROW.

NOTES

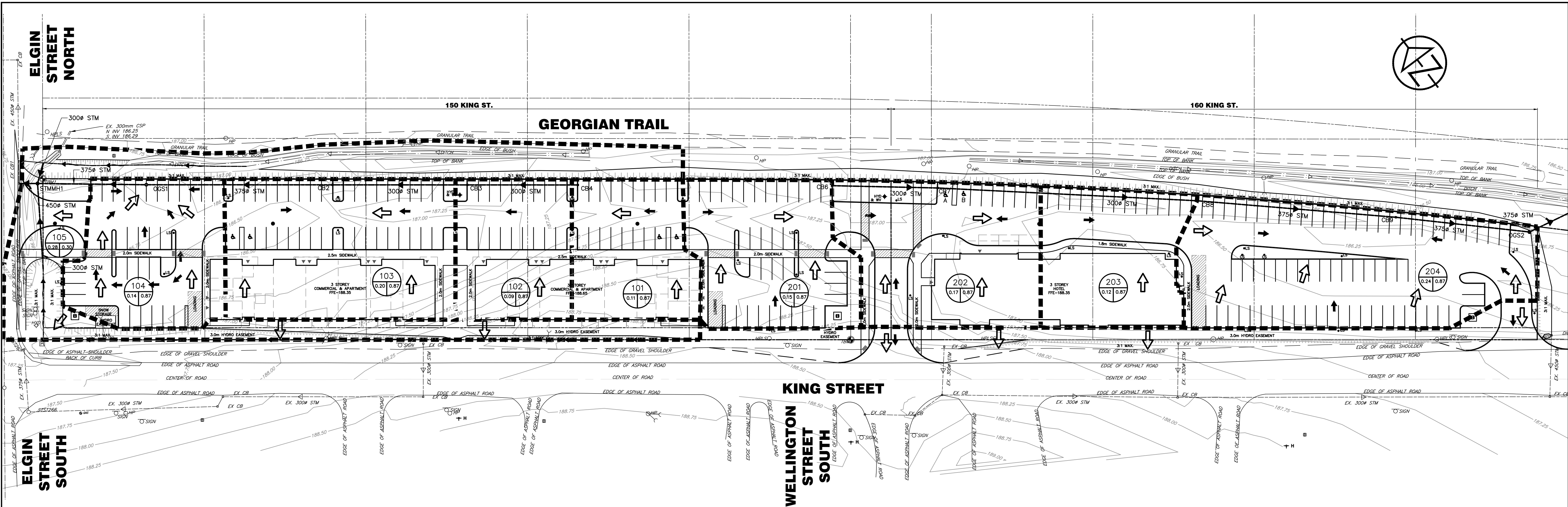
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No.	REVISION DESCRIPTION	DATE	ENGINEER STAMP



150/160 KING STREET THORNBURY TOWN OF THE BLUE MOUNTAINS				TATHAM ENGINEERING			
SILTATION & EROSION CONTROL PLAN				DESIGN: RD	FILE: 122030	DWG:	SC-1
				DRAWN: RD	DATE: APR 2022		
				CHECK: RS	SCALE: 1:500		



Storm Sewer Design Sheet

Version Number: 1 Version Date: May 30, 2022

LEGEND

- CATCHMENT ID: 201
- CATCHMENT AREA (ha): 13.6 | 0.50
- RUNOFF COEFFICIENT: 0.12 | 0.87
- CATCHMENT BOUNDARY: Dashed line
- PROPOSED MINOR OVERLAND FLOW: Arrow with 'M'
- PROPOSED MAJOR OVERLAND FLOW: Arrow with 'O'
- PROPOSED GRASSED SWALE: Arrow with 'S'
- EXISTING CONTOUR/ELEVATION: Dotted line

Project Information	
150 & 160 King Street East, Thornbury	122030
Drawing Reference	
SWM01	May 30-22
Prepared By	
John Birchard	May 30-22
Reviewed By	
Andrew Schoof	May 30-22

Municipality		
Town of The Blue Mountains		
Runoff Coefficient Adjustment		
Year	A	B
10	0.80	0.20
25	0.70	0.30
50	0.60	0.40
100	0.50	0.50

Manning's Coefficient	
Pipe	Value
CSP	0.024
Concrete	0.013
PVC	0.009
Time of Concentration	
15 mins	

IDF Curve Coefficients			
Year	A	B	C
2	807.44	6.75	0.83
5	1135.40	7.50	0.84
10	1387.00	7.97	0.85
25	1676.20	8.30	0.86
50	1973.10	9.00	0.87
100	2193.10	9.04	0.87

Engineer Stamp													

Street Name	Area ID / Label	Upstream Maintenance Hole	Downstream Maintenance Hole	Area (ha)	5 Year Runoff Coefficient	Design Storm (Year)	Adjusted Runoff Coefficient	Area x Runoff Coefficient	Cumulative Area (ha)	Cumulative Area x Adjusted Runoff Coefficient	Time of Concentration (min)	Rainfall Intensity (mm/hr)	Peak Flow (m³/s)	Manning's Roughness Coefficient	Sewer Length (m)	Sewer Slope (%)	Actual Sewer Diameter (mm)	Full Flow Velocity (m/s)	Full Flow Capacity (L/s)	Actual Velocity (m/s)	Travel Time (min)	Calculated Sewer Diameter (mm)	Percentage of Full Flow Capacity (%)	Total Time of Travel (min)
West Outlet	101	CB4	CB3	0.11	0.87	5	0.87	0.10	0.11	0.10	15.00	82.79	0.023	0.013	27.5	0.5%	300	0.97	0.068	0.81	0.57	198	33.1%	15.57
	102	CB3	CB2	0.09	0.87	5	0.87	0.08	0.20	0.18	15.57	81.07	0.040	0.013	33.6	0.5%	300	0.97	0.068	0.94	0.60	245	58.2%	16.16
	103	CB2	OGS1	0.20	0.87	5	0.87	0.17	0.40	0.35	16.16	79.35	0.077	0.013	46.4	0.5%	375	1.12	0.124	1.11	0.70	314	62.3%	16.86
	104	OGS1	STMMH1	0.14	0.87	5	0.87	0.12	0.54	0.47	16.86	77.44	0.102	0.013	27.1	0.5%	375	1.12	0.124	1.12	0.40	348	82.0%	17.26
	105	STMMH1	EXCB1	0.28	0.87	5	0.87	0.24	0.82	0.72	17.26	76.38	0.152	0.013	4.7	0.5%	450	1.27	0.202	1.27	0.06	405	75.4%	17.32
East Outlet	201	CB6	CB7	0.15	0.87	5	0.87	0.13	0.15	0.13	15.00	82.79	0.030	0.013	26.3	0.5%	300	0.97	0.068	0.87	0.50	220	43.9%	15.50
	202	CB7	CB8	0.17	0.87	5	0.87	0.15	0.32	0.28	15.50	81.26	0.063	0.013	64.6	0.5%	300	0.97	0.068	0.97	1.11	291	91.9%	16.62
	203	CB8	CB9	0.12	0.87	5	0.87	0.10	0.44	0.38	16.62	78.09	0.083	0.013	49.7	0.5%	375	1.12	0.124	1.12	0.74	323	67.0%	17.35
	204	CB9	OGS2	0.24	0.87	5	0.87	0.21	0.68	0.59	17.35	76.14	0.125	0.013	33.4	0.5%	375	1.17	0.129	1.17	0.47	370	96.7%	17.83
	-	OGS2	OUTLET	0.00	0.87	5	0.87	0.00	0.68	0.59	17.83	74.94	0.123	0.013	12.0	0.5%	375	1.12	0.124	1.12	0.18	374	99.3%	18.01

DISCLAIMER AND COPYRIGHT

CONTRACTOR MUST VERIFY ALL DIMENSIONS AND BE RESPONSIBLE FOR SAME. ANY DISCREPANCIES MUST BE REPORTED TO THE ENGINEER BEFORE COMMENCING WORK. DRAWINGS ARE NOT TO BE SCALED.

TATHAM ENGINEERING LIMITED CLAIMS COPYRIGHT TO THIS DRAWING WHICH MAY NOT BE USED FOR ANY PURPOSE OTHER THAN THAT PROVIDED IN THE CONTRACT BETWEEN THE OWNER/CLIENT AND THE ENGINEER WITHOUT THE EXPRESS CONSENT OF TATHAM ENGINEERING LIMITED.

BENCHMARKS

TBM1 – ELEVATION 186.52
ELEVATIONS SHOWN HEREON ARE GEODETIC AND REFER TO THE TOP OF IRON BAR, NORTH WEST CORNER OF LOT.

TBM2 – ELEVATION 185.37
ELEVATIONS SHOWN HEREON ARE GEODETIC AND REFER TO THE TOP OF IRON BAR, NORTH EAST CORNER OF LOT.

TBM3 – ELEVATION 188.63
ELEVATIONS SHOWN HEREON ARE GEODETIC AND REFER TO THE TOP OF IRON BAR, SOUTH WEST CORNER OF WELLINGTON STREET ROW.

NOTES

LEGAL SURVEY INFORMATION AND LOT DIMENSIONS SHOWN ON THIS PLAN ARE TAKEN FROM A SURVEY PLAN PREPARED BY JO BARNES LTD DATED FEBRUARY 25, 2022, WHICH MAY NOT BE FINAL AND ARE NOT GUARANTEED. THE FINAL REGISTERED PLAN SHALL BE REFERRED TO FOR CONFIRMATION OF THE DATA.

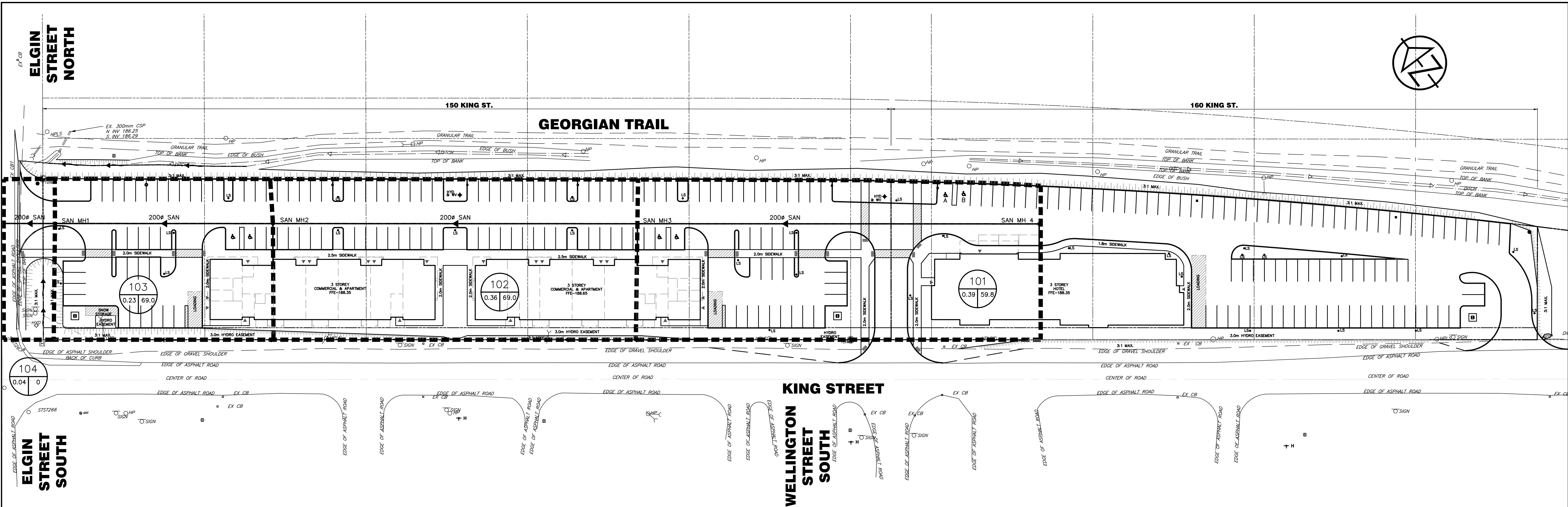
TOPOGRAPHIC INFORMATION SHOWN ON THIS PLAN IS FROM A SURVEY PREPARED BY JOETOPO SURVEYS AND CADD INC, DATED MARCH 28, 2020.

No.	REVISION DESCRIPTION	DATE	ENGINEER STAMP

150/160 KING STREET
THORNBURY
TOWN OF THE BLUE MOUNTAINS

STORM DRAINAGE
AREA PLAN

DESIGN: RD	FILE: 122030	DWG: STM-1
DRAWN: RD	DATE: APR 2022	
CHECK: RS	SCALE: 1:500	



Sanitary Sewer Design Sheet

Version Number: 1 Version Date: May 30, 2022

LEGEND

SANITARY AREA BOUNDARY

AREA IDENTIFICATION NUMBER

AREA IN HECTARES

POPULATION (CAP.)

Project Information	
150 & 160 King Street East, Thornbury	122030
Drawing Reference	
SAN01	May 30-22
Prepared By	
John Birchard	May 30-22
Reviewed By	
Andrew Schoof	May 30-22

Municipality			
Town of The Blue Mountains			
Population Density			
Capita per Unit	Low	Medium	High
	2.30	2.30	2.30
Infiltration			
Infiltration (L/s/ha)	0.23		

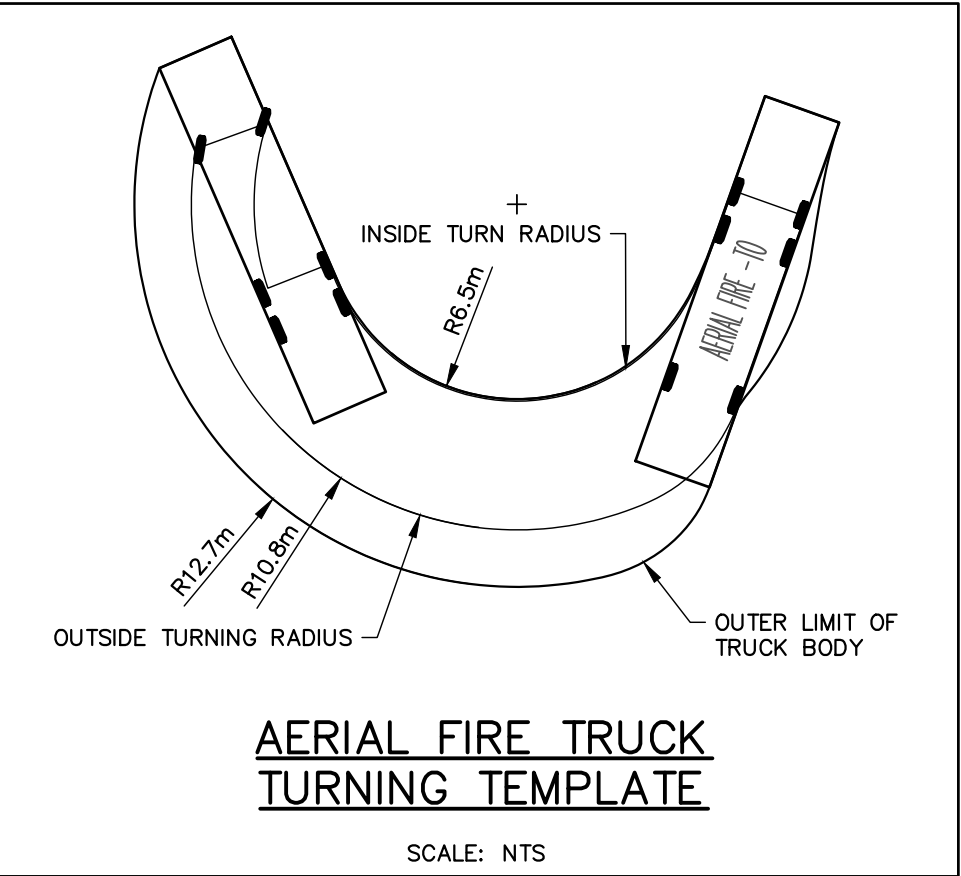
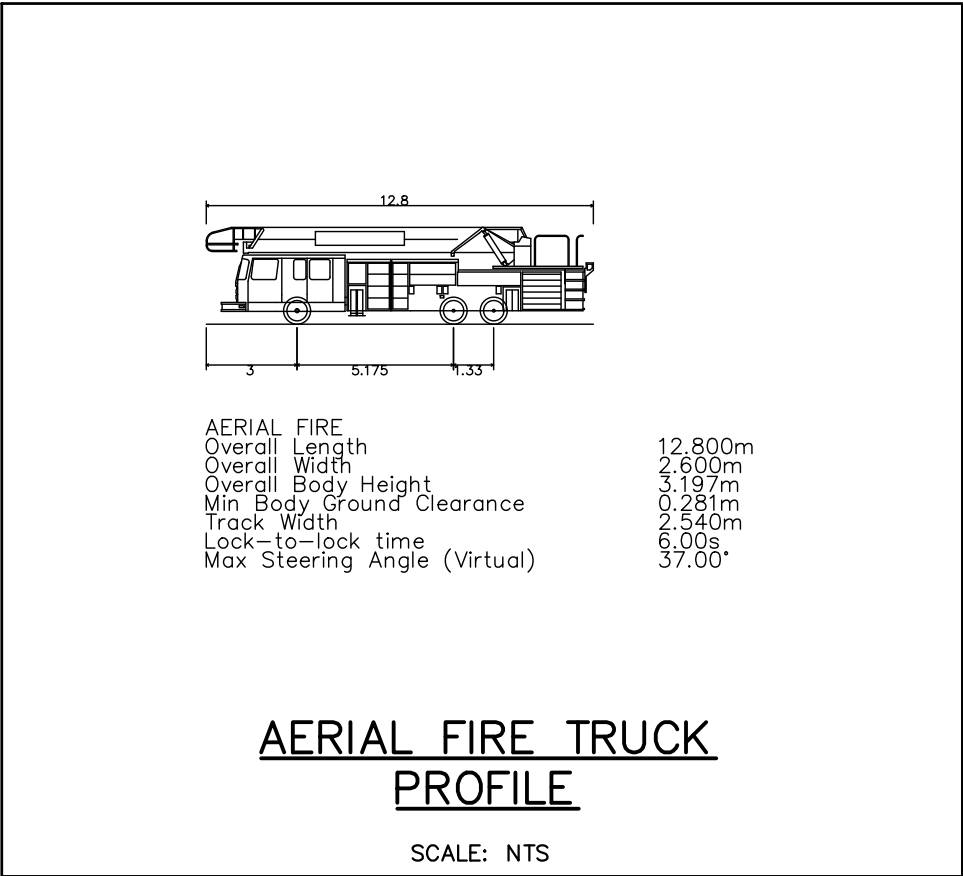
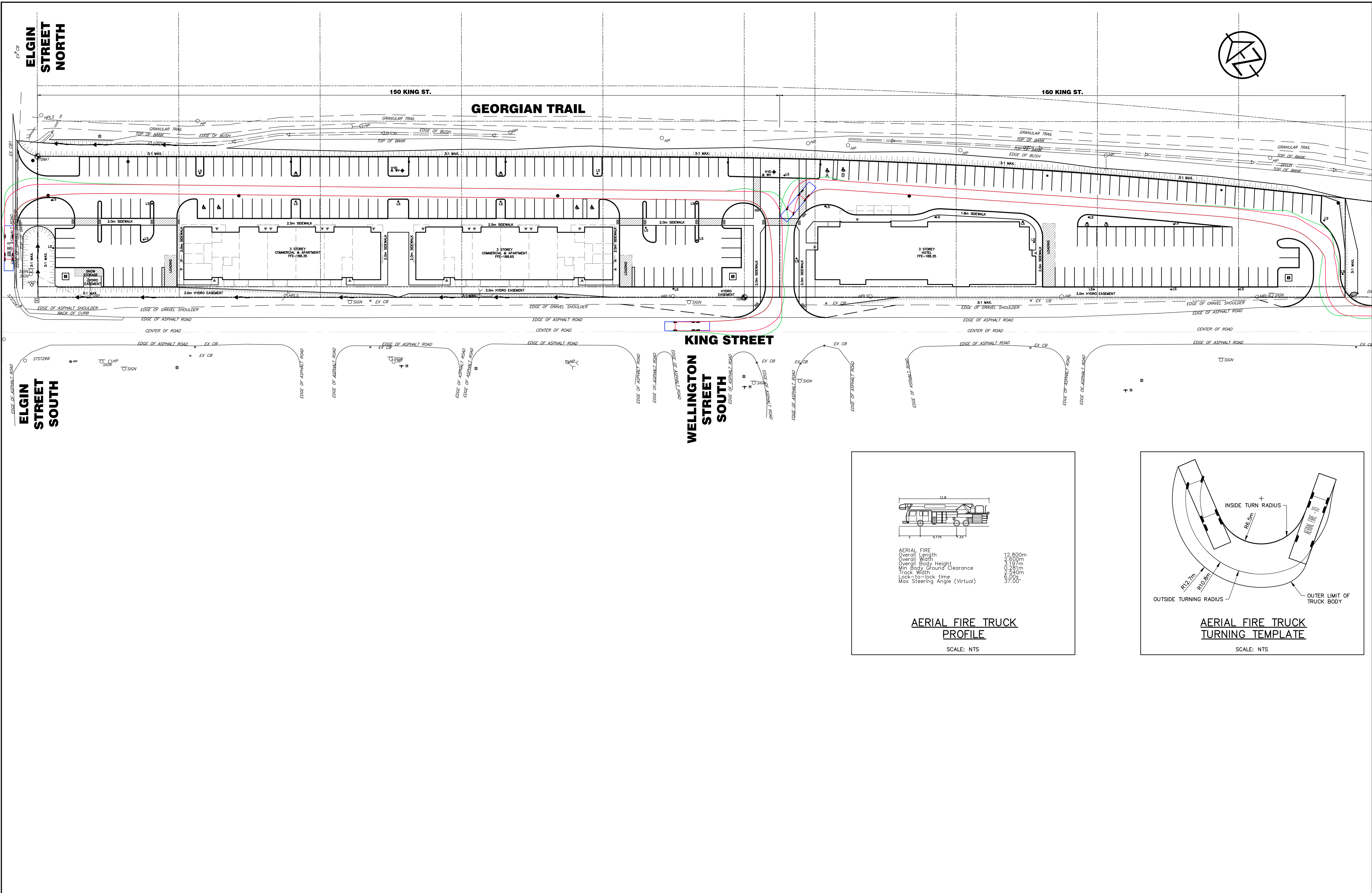
Flow		
Development Type	Average (L/cap/day)	Peaking Factor
Residential	450	Harmon
Development Type		
Average (L/ha/day)		
Institution	-	-
Commercial	-	-
Industrial High Intensity	55,000	-
Industrial Low Intensity	20,000	-

Manning's Coefficient	
Pipe Material	Value
Concrete	0.013
PVC	0.013
Applied	0.013

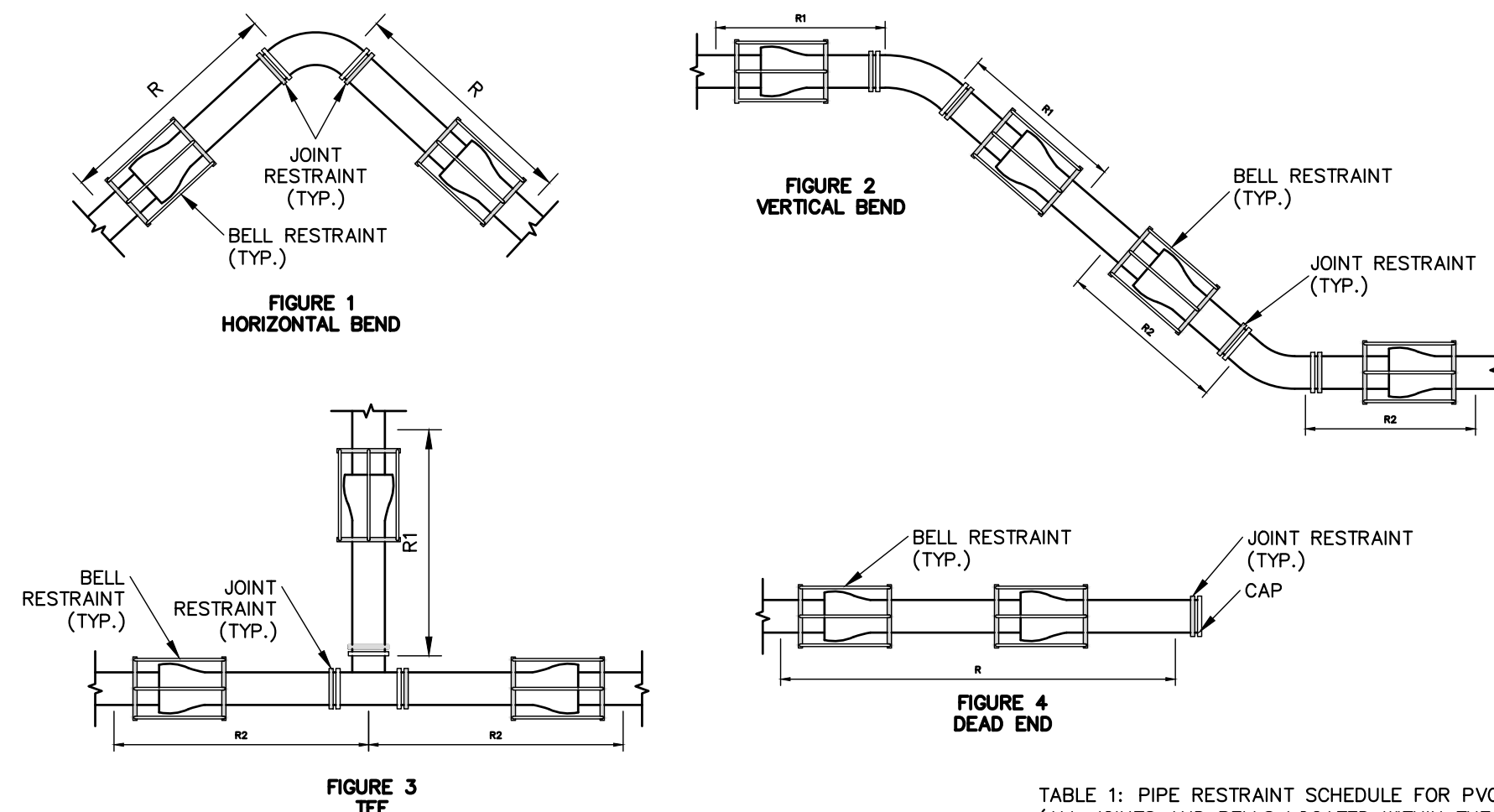
Engineer Stamp

Area Description	Area Label/ID	Upstream Maintenance Hole	Downstream Maintenance Hole	Development Type	Population Density	Number of Units	Population (cap)	Accumulated Population (cap)	Peaking Factor	Area (ha)	Cumulative Area (ha)	Average Flow (L/s)			Peak Flow (L/s)			Proposed Sanitary Sewer							
												Development	Infiltration	Total	Development	Infiltration	Total	Sewer Length (m)	Sewer Slope (%)	Actual Sewer Diameter (mm)	Full Flow Velocity (m/s)	Full Flow Capacity (L/s)	Actual Velocity (m/s)	Calculated Sewer Diameter (mm)	Percentage of Full Flow Capacity (%)
Hotel	101	MH4	MH3	Residential	High	26	59.8	59.8	4.30	0.39	0.39	0.31	0.09	0.40	1.34	0.09	1.43	100.0	1.0%	200	1.04	32.80	0.52	62	4.4%
East Apartment	102	MH3	MH2	Residential	High	30	69.0	128.8	4.21	0.36	0.75	0.67	0.17	0.84	2.85	0.17	3.02	90.7	0.5%	200	0.74	23.19	0.49	93	13.0%
West Apartment	103	MH2	MH1	Residential	High	30	69.0	197.8	4.15	0.23	0.98	1.03	0.23	1.26	4.34	0.23	4.57	57.3	0.5%	200	0.74	23.19	0.55	109	19.7%
None	104	MH1	MAIN	Residential	High	0	0.0	197.8	4.15	0.04	1.02	1.03	0.23	1.26	4.34	0.23	4.58	9.7	0.5%	200	0.74	23.19	0.55	109	19.7%

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CHARACTERISTICS USED TO CALCULATE RESTRAINT LENGTH:

PIPE MATERIAL: PVC
 SOIL TYPE: ML (SILTS, VERY FINE SANDS, SILTY OR CLAYEY FINE SANDS)
 SAFETY FACTOR: 1.5 TO 1
 TRENCH TYPE: 5 (PIPE BEDDED IN COMPACT GRANULAR MATERIAL)
 DEPTH OF BURY: 1.8m (6 ft)
 TEST PRESSURE: 1035 kPa (150 p.s.i.)
 PROGRAM: SIGMA IRON – RESTRAINT LENGTH CALCULATOR 1.0

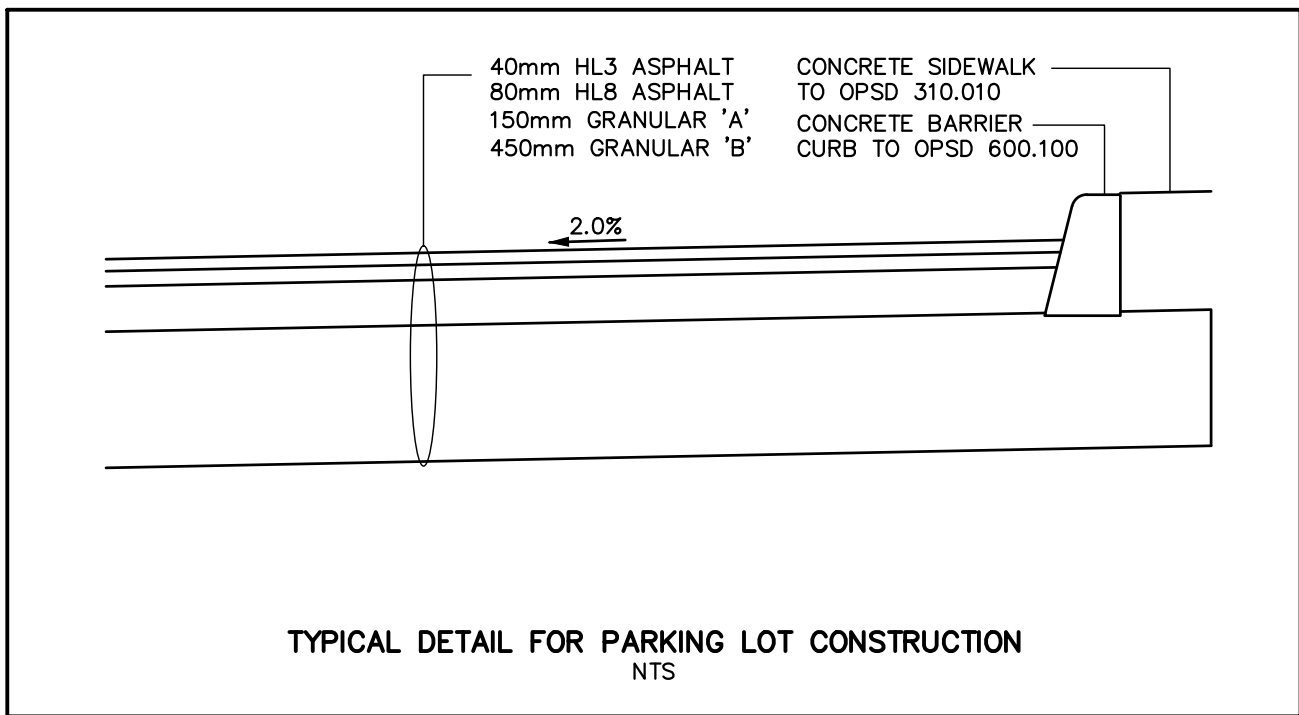
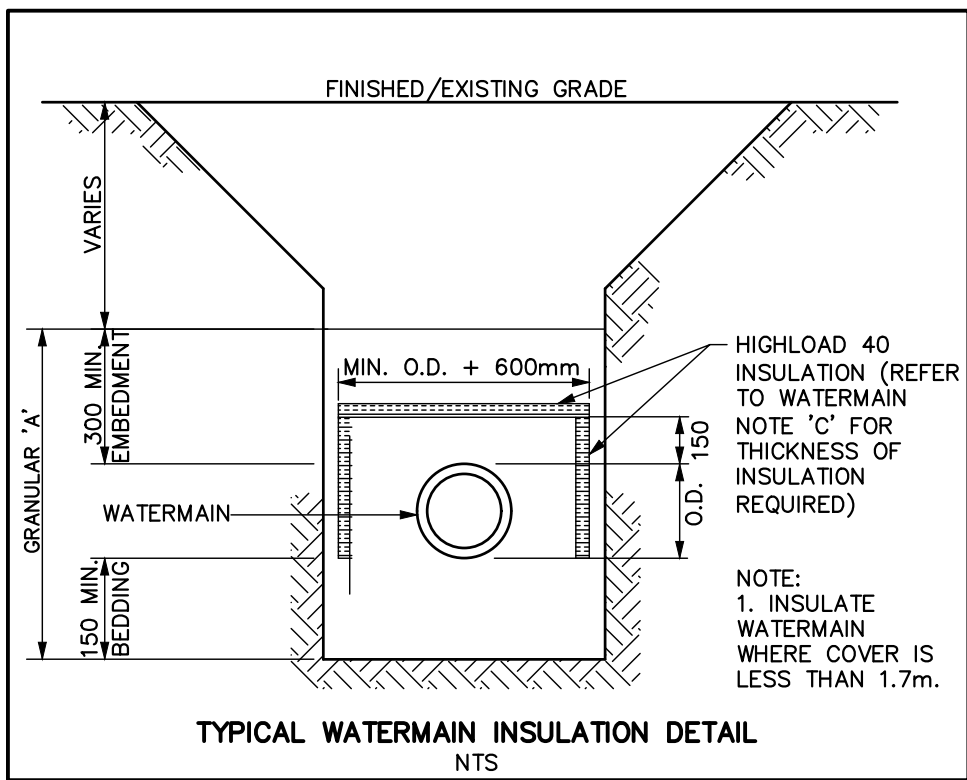
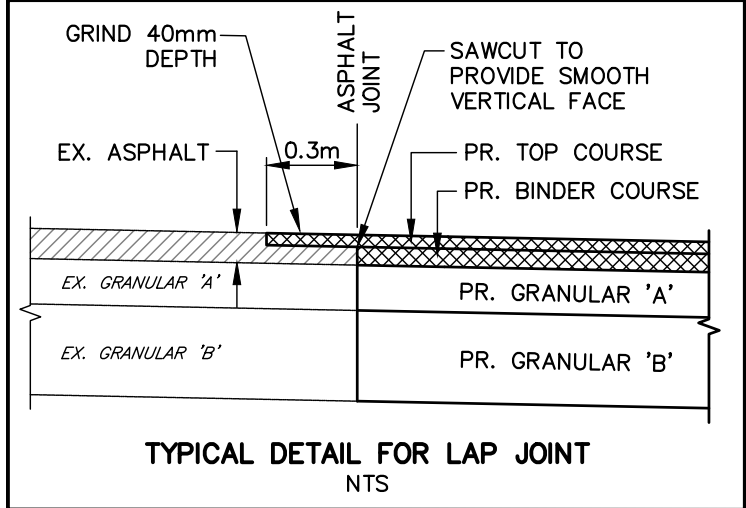
TABLE 2: PIPE RESTRAINT SCHEDULE FOR PVC TEES
 (ALL JOINTS AND BELLS LOCATED WITHIN THE CALCULATED RESTRAINT LENGTH SHALL BE RESTRAINED WITH APPROVED BELL AND JOINT RESTRAINTS)

TEES			
NOMINAL PIPE DIA.	BRANCH PIPE DIA.	RESTRAINT 1 LENGTH (m)	RESTRAINT 2 LENGTH (m)
150	150	4.6	3.0
200	150	2.4	3.0
150	200	9.8	3.0
200	200	7.9	3.0
300	150	0.3	3.0
300	200	4.0	3.0
300	300	14.3	3.0

TABLE 1: PIPE RESTRAINT SCHEDULE FOR PVC PIPE
 (ALL JOINTS AND BELLS LOCATED WITHIN THE CALCULATED RESTRAINT LENGTH SHALL BE RESTRAINED WITH APPROVED BELL AND JOINT RESTRAINTS)

PIPE DIA.	BEND ANGLE	HORIZONTAL BEND	VERTICAL BEND		DEAD END (NO BEND ANGLE)
		RESTRAINT LENGTH (m)	RESTRAINT 1 LENGTH (m)	RESTRAINT 2 LENGTH (m)	RESTRAINT LENGTH (m)
150Ø	90°	3.7	—	—	11.6
	45°	1.6	4.9	1.5	
	22.5°	0.9	2.5	0.9	
	11°	0.6	1.3	0.6	
200Ø	90°	4.6	—	—	15.0
	45°	2.2	6.1	2.2	
	22.5°	0.9	3.1	1.0	
	11°	0.6	1.6	0.6	
300Ø	90°	8.4	—	—	21.4
	45°	2.8	8.9	2.8	
	22.5°	1.5	4.3	1.5	
	11°	0.9	3.4	0.9	

- NOTES:
- CONTRACTOR TO REPORT IN WRITING TO THE ENGINEER ANY CHANGES TO SOIL OR SITE CHARACTERISTIC THAT MAY ALTER THE PIPE RESTRAINT CALCULATION.
 - THE CONTRACTOR IS RESPONSIBLE TO CONFIRM THRUST RESTRAINT REQUIREMENTS WITH THE PIPE AND RESTRAINT MANUFACTURERS.
 - VALVES TO BE RESTRAINED AS IF THEY ARE DEAD ENDS.



GENERAL

- ALL WORK TO BE CARRIED OUT IN ACCORDANCE WITH TOWN OF THE BLUE MOUNTAINS STANDARDS AND OPS STANDARDS. WHERE CONFLICT OCCURS, TOWN STANDARDS TO GOVERN.
- THE CONTRACTOR MUST OBTAIN AN ENCROACHMENT PERMIT FROM THE COUNTY OF GREY FOR ALL WORKS WITHIN THE COUNTY ROAD ALLOWANCE PRIOR TO THE COMMENCEMENT OF CONSTRUCTION.
- THE OWNER'S ENGINEER SHALL PROVIDE BENCHMARK ELEVATIONS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DETAILED LAYOUT OF THE WORK.
- ALL PROPERTY BARS TO BE PRESERVED AND REPLACED BY AN OLS AT CONTRACTOR'S EXPENSE IF REMOVED/DISTURBED DURING CONSTRUCTION.
- THE CONTRACTOR SHALL MAKE ARRANGEMENTS FOR THE SUPPLY OF TEMPORARY WATER AND POWER. DEWATERING TO BE CARRIED OUT IN ACCORDANCE WITH OPSS.MUNI 517 AND OPSS.MUNI 518. MAINTAIN ALL TRENCHES IN A DRY CONDITION. A MOECC PERMIT TO TAKE WATER (PTTW) HAS BEEN OBTAINED FOR THIS PROJECT. THE CONTRACTOR SHALL ENSURE THAT ALL DEWATERING WORKS ARE IN ACCORDANCE WITH THE PERMIT.
- ALL ENGINE DRIVEN PUMPS TO BE ADEQUATELY SILENCED, SUITABLE FOR OPERATION IN A RESIDENTIAL DISTRICT.
- GENERAL INSTALLATION AND TESTING OF SEWERS, WATERMAIN AND APPURTENANCES TO BE IN ACCORDANCE WITH OPSS 407, 408, OPSS.MUNI 409, OPSS.MUNI 410, OPSS.MUNI 421 AND OPSS.MUNI 441, AND ALL SPECIFICATIONS REFERENCED WITHIN THESE SECTIONS.
- ALL MAINTENANCE HOLES ARE 1200 mm DIAMETER, UNLESS OTHERWISE SPECIFIED.
- ALL STRUCTURES TO BE INSTALLED WITH FROST STRAPS TO OPSS 701.100.
- PIPE SUPPORT AT ALL STRUCTURES TO OPSS 708.020.
- ALL MAINTENANCE HOLE AND CATCH BASIN FRAME AND GRATES TO BE SET TO FINAL COURSE ASPHALT ELEVATION IN ACCORDANCE WITH OPSS 704.010, USING CONCRETE ADJUSTMENT UNITS. MAXIMUM COMBINED HEIGHT OF ADJUSTMENT UNITS SHALL BE 450 mm.
- TRENCH BACKFILL TO BE SELECT NATIVE MATERIAL OR IMPORTED SELECT SUBGRADE MATERIAL TO OPSS.MUNI 1010. BACKFILL TO BE PLACED IN MAXIMUM 200 mm THICK LIFTS (OR AS OTHERWISE DIRECTED BY THE GEOTECHNICAL CONSULTANT) AND COMPACTED TO A DRY DENSITY OF AT LEAST 95% OF THE MATERIAL'S STANDARD PROCTOR MAXIMUM DRY DENSITY (SPMDD).
- PIPE EMBEDMENT TO BE COMPACTED TO A DRY DENSITY OF AT LEAST 95% OF THE MATERIAL'S SPMDD. BACKFILL AND EMBEDMENT TO OPSS 802.010 (FLEXIBLE PIPE), GRANULAR 'A' EMBEDMENT OR OPSS 802.031 (RIGID PIPE) CLASS 'B' GRANULAR 'A' BEDDING, GRANULAR 'B' COVER (MAX. AGGREGATE SIZE 25 mm), MINIMUM BEDDING DEPTH 150 mm, MINIMUM COVER DEPTH 300 mm ON ALL PIPES. WHERE EXCESSIVELY WET OR POOR SUBGRADE IS ENCOUNTERED AT THE INVERT LEVEL, IT MAY BE NECESSARY TO INCREASE THE BEDDING THICKNESS.
- CLEAR STONE COMPLETELY WRAPPED IN FILTER FABRIC MAY BE SUBSTITUTED FOR EMBEDMENT MATERIAL IF APPROVED BY THE ENGINEER.
- DISTURBED AREAS TO BE REINSTATED TO PREVIOUS CONDITION OR BETTER.
- REINSTATEMENT OF ALL DISTURBED BOULEVARDS AND DITCHES TO INCLUDE REGRADING, PLACEMENT OF 150mm TOPSOIL AND SEED/SOD IN ACCORDANCE WITH OPSS.MUNI 802 AND OPSS.MUNI 803. SOD TO BE STAKED WHERE NECESSARY TO AVOID MOVEMENT.
- THE CONTRACTOR IS RESPONSIBLE FOR THE PRESERVATION OF ALL EXISTING INFRASTRUCTURE/FACILITIES AS WELL AS NOTIFYING ALL UTILITY COMPANIES PRIOR TO COMMENCING WORK AND CO-ORDINATE CONSTRUCTION ACCORDINGLY.
- ALL ON-SITE MATERIAL SHALL BE PROPERLY STORED, SECURED, MONITORED AND COVERED AS REQUIRED. SPECIFICALLY, ALL PVC PIPE SHALL BE COVERED WHILE STORED ON-SITE.
- ALL SIGNAGE TO COMPLY WITH THE TOWN OF THE BLUE MOUNTAINS SIGN BY-LAW.
- HAZARDOUS TREE REMOVAL IS TO BE COMPLETED ALONG ANY WALKING PATHS OR TRAILS, LEANING OR BACKING ONTO INDIVIDUAL LOTS, OR ADJACENT TO ANY DITCHES OR ANY PROPOSED DITCHES TO A MINIMUM OF 6.0m FROM PROPERTY LINES, TRAILS, AND DITCHES. TREES THAT HAVE BEEN UNDERMINED OR HAVE HAD ITS ROOTS EXPOSED DURING CONSTRUCTION MAY BECOME A HAZARDOUS TREE AND MUST ALSO BE REMOVED.
- THE MINIMUM COVER OVER ALL CULVERTS SHALL BE 0.3m IN ACCORDANCE WITH TOWN STANDARDS OR AS REQUIRED IN ACCORDANCE WITH MANUFACTURING GUIDELINES. THE GREATER SHALL GOVERN.
- THE MINIMUM COVER OVER ALL UTILITIES SHALL BE 0.9m IN ACCORDANCE WITH TOWN STANDARDS OR AS REQUIRED BY UTILITY PROVIDER. THE GREATER SHALL GOVERN.

ROADS

- SUBGRADE AND BOULEVARD MATERIAL TO BE COMPACTED TO A DRY DENSITY OF AT LEAST 95% OF THE MATERIAL'S SPMDD. SUBGRADE TO BE PROOF ROLLED WITH THE TOWN AND GEOTECHNICAL REPRESENTATIVE PRESENT PRIOR TO PLACING GRANULAR 'B'.
- LIMESTONE SCREENINGS, 150mm GRANULAR 'A' AND 400mm 'B' TO BE COMPACTED TO A DRY DENSITY OF AT LEAST 100% OF THE MATERIAL'S RESPECTIVE SPMDD.
- ASPHALT TO BE 40mm HL3 SURFACE, 80mm HL8 BASE TO BE COMPACTED TO A MINIMUM OF 92% OF THE MARSHALL MAXIMUM RELATIVE DENSITY (M.R.D.).
- ROADWAYS WITHIN MUNICIPAL RIGHT-OF-WAY TO BE CONSTRUCTED IN ACCORDANCE WITH THE APPLICABLE ROAD CROSS-SECTIONS.
- JOINTS WITH EXISTING ASPHALT TO BE SAW CUT STRAIGHT PRIOR TO PLACING NEW ASPHALT. WHERE EXISTING ASPHALT IS THICKER THAN 75 mm, A 300 mm WIDE LAP JOINT SHALL BE GROUND INTO EXISTING ASPHALT PRIOR TO THE PLACEMENT OF SURFACE COURSE ASPHALT (SEE DETAIL).
- ALL GRANULAR AND ASPHALT MATERIAL TO BE PLACED IN ACCORDANCE WITH OPSS.MUNI 310 AND OPSS.MUNI 314.
- STOP SIGNS AND STREET SIGNS TO BE SUPPLIED AND INSTALLED PER TOWN STANDARDS.
- TACK COAT TO BE APPLIED BETWEEN BASE AND TOP COURSE ASPHALT.
- CONCRETE BARRIER CURB TO OPSS 600.110, AND OPSS.MUNI 353. CURB DEPRESSIONS AT SIDEWALK CROSSINGS IN ACCORDANCE WITH OPSS 310.033. DEPRESSIONS AT DRIVEWAYS IN ACCORDANCE WITH OPSS 351.010. THE MAXIMUM WIDTH SHALL BE IN ACCORDANCE WITH TOWN BYLAW 2014-48.
- CONCRETE CURB TERMINATIONS TO BE CONSTRUCTED IN ACCORDANCE WITH OPSS 608.010.
- CONCRETE SIDEWALK TO OPSS 310.010, CONCRETE SIDEWALK RAMPS TO OPSS 310.033 AND OPSS.MUNI 351. SUBBASE TO CONSIST OF 150 mm DEPTH GRANULAR 'A'.
- UNPAINTED CAST IRON TACTILE WALKING SURFACE INDICATOR COMPONENTS TO BE IN ACCORDANCE WITH OPSS 310.039.
- 100 mm DIAMETER PIPE SUBDRAINS SHALL BE PROVIDED ON BOTH SIDES OF THE ROAD IN ACCORDANCE WITH OPSS.MUNI 405 AND OPSS 216.021, UNWRAPPED TRENCH, GRANULAR 'A' EMBEDMENT.
- SUBDRAINS TO BE PERFORATED, COMPLETE WITH FILTER SOCK, OTHER THAN THE 2.0 m SECTION IMMEDIATELY UPSTREAM OF ALL STRUCTURES WHICH SHALL BE NON-PERFORATED.
- ALL ROADWAY CULVERTS TO HAVE FROST TREATMENT AS PER OPSS 803.031.
- SURFACE ASPHALT SHALL BE COMPLETED IN A SINGLE MOBILIZATION FOR THE ENTIRE PHASE.

STORM SEWER

- MAINTENANCE HOLES AND CATCH BASIN MAINTENANCE HOLES TO OPSS 701.010, 701.011, 701.012.
- MAINTENANCE HOLES TO BE BENCHMARKED TO OPSS 701.021. MINIMUM BENCHMARKING WIDTH SHALL BE 230 mm.
- CATCH BASIN MAINTENANCE HOLES TO HAVE A SUMP PER OPSS 701.010.
- MAINTENANCE HOLE AND CATCH BASIN MAINTENANCE HOLE STEPS TO OPSS 405.010.
- MAINTENANCE HOLE FRAMES AND GRATES TO OPSS 401.010 TYPE 'A' CLOSED COVER.
- CATCH BASINS TO OPSS 705.010 AND 705.020. ROADSIDE CATCH BASINS TO HAVE A SUMP.
- DITCH INLET CATCH BASINS TO OPSS 705.030. DITCH INLET CATCH BASIN GRATES TO OPSS 403.010, 3:1 GRATE UNLESS OTHERWISE SPECIFIED.
- CATCH BASIN AND CATCH BASIN MAINTENANCE HOLE FRAMES AND GRATES TO OPSS 400.020. FRAME AND GRATES TO BE INSTALLED IN THE CURB LINE TO OPSS 610.010.
- CATCH BASIN LEADS – 300 mm DIAMETER SINGLE AND 375 mm DIAMETER DOUBLE. CATCH BASIN CONNECTIONS TO OPSS 708.010 AND OPSS 708.030.
- ALL ROADWAY CULVERTS TO HAVE FROST TREATMENT AS PER OPSS 803.031.

SANITARY SEWER

- MAINTENANCE HOLES TO OPSS 701.010.
- MAINTENANCE HOLES TO BE BENCHMARKED TO OPSS 701.021 ALL CONNECTIONS TO MAINTENANCE HOLES TO INCLUDE KOR-N-SEAL RUBBER BOOT PIPE CONNECTION.
- MAINTENANCE HOLE STEPS TO OPSS 405.010.
- MAINTENANCE HOLE FRAME AND COVER TO OPSS 401.010 TYPE 'A' CLOSED COVER. WHERE SPECIFIED, WATER TIGHT FRAME AND GRATE TO OPSS 401.050 SHALL BE USED.

WATERMAIN

- ALL WORK ON EXISTING WATERMAIN TO BE COORDINATED WITH THE TOWN. TOWN TO BE PROVIDED WITH PRELIMINARY SCHEDULE AND 48 HOURS NOTICE.
- FIRE SERVICE MAINS AND WATER SERVICE PIPES COMBINED WITH FIRE SERVICE MAINS ARE REQUIRED TO CONFORM WITH NFPA 24 "INSTALLATION OF PRIVATE FIRE SERVICE MAINS AND THEIR APPURTENANCES" AS PER SENTENCE 7.2.11.1(1), DIVISION 'B' OF THE ONTARIO BUILDING CODE.
- FIRE DEPARTMENT CONNECTIONS TO INCLUDE TWO 65 mm HOSE CONNECTIONS WITH FEMALE SWIVEL HOSE COUPLINGS MOUNTED NOT LESS THAN 300 mm AND NOT MORE THAN 900 mm ABOVE GROUND LEVEL IN ACCORDANCE WITH SECTION 3.2.5.16, DIVISION 'B' OF THE ONTARIO BUILDING CODE.
- ALL FIRE LINES AND SERVICE LINES TO INCLUDE THE INSTALLATION OF AN INDIVIDUAL PRV WITHIN EACH BUILDING. PRV TO BE SUPPLIED AND INSTALLED BY THE BUILDER DURING CONSTRUCTION.
- MINIMUM GROUND COVER OVER WATERMAIN, SERVICE LATERALS AND HYDRANT LEADS TO BE 1.7m AT ALL POINTS, MAXIMUM GROUND COVER OVER WATERMAIN TO BE 2.5 m.
- INSULATION TO BE INSTALLED OVER WATERMAIN OR SERVICES WITH LESS THAN 1.7m DEPTH OF COVER AS NOTED ON THE DRAWINGS OR MINIMUM AS FOLLOWS:
 G. DEPTH OF COVER BETWEEN 1.4 TO 1.7m REQUIRES 50mm INSULATION.
 H. DEPTH OF COVER BETWEEN 1.2 TO 1.7m REQUIRES 100mm INSULATION.
 I. WATERMAIN NOT TO BE INSTALLED WITH LESS THAN 1.2 METRES DEPTH OF COVER.
- PIPE RESTRAINTS TO BE PROVIDED AT ALL CHANGES IN PIPE DIRECTION, TERMINATIONS AND ANY LOCATION WHERE THRUST PRESSURES MAY OCCUR. WATER VALVES SHALL BE RESTRAINED ON EITHER SIDE TO THE SAME STANDARD THAT A DEAD END WOULD BE. WHERE SOIL CONDITIONS ARE SUSPECT, SUCH AS IN DISTURBED SOILS OR SOILS WITH BEARING STRENGTH OF LESS THAN 200 kPa, SIGMA SLCS SHALL BE USED. IN LIEU OF THRUST BLOCKS PIPE RESTRAINTS FOR PVC SHALL BE PER TOWN STANDARDS, INSTALLED IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS (SEE CHART). THREADED ROD IN JOINT RESTRAINT IS NOT PERMISSIBLE.
- CATHODIC PROTECTION OF ALL WATERMAIN FITTINGS AND APPURTENANCES TO BE PROVIDED AS PER TOWN STANDARD (SEE CHART).
- HYDRANTS TO BE INSTALLED TO OPSS 1105.010.
- HYDRANTS TO BE PAINTED CHROME YELLOW AND INCLUDE A FLEXSTAKE HYDRANT MARKER MODEL FHV804, 1.2 m LONG, COLOUR YELLOW WITH REFLECTIVE GRAPHIC ON BOTH SIDES AT THE TOP OF THE MARKER. MARKER TO BE POSITIONED ON THE RIGHT PORT AS VIEWED FROM THE STREET. A FIRE HYDRANT MARKER POST AND SIGN SHALL BE INSTALLED 0.3 m BEHIND EACH HYDRANT. THE SIGN SHALL BE REFLECTIVE WITH A RED HYDRANT ON A WHITE BACKGROUND AND MEASURE 0.3 x 0.3 m (OR APPROVED EQUAL). THE SIGN SHALL BE MOUNTED 1.5 m ABOVE GRADE.
- ALL PVC WATERMAIN AND FIRE LINES TO HAVE TRACER WIRE BETWEEN HYDRANTS AND OTHER CONDUCTING APPURTENANCES. CONTRACTOR SHALL TEST ALL TRACER WIRE TO CONFIRM CONNECTIVITY.
- TRACER WIRE TO BE 10 GAUGE, MULTI-STRAND SHALL BE PLACED ON TOP AND ATTACHED IN TWO PLACES ON EACH LENGTH OF PVC OR PE PIPE. ALL CONNECTIONS SHALL BE MADE WITH "DRYCONN WATERPROOF CONNECTORS" OR APPROVED EQUAL.
- VERTICAL CLEARANCE BETWEEN WATERMAINS AND SEWERS TO BE A MINIMUM OF 0.5m WHEN WATERMAIN IS BELOW THE SEWERS, HORIZONTAL CLEARANCE BETWEEN WATERMAINS AND SEWERS TO BE A MINIMUM OF 2.5m.
- THE COMPLETE WATER SYSTEM, INCLUDING SERVICE CONNECTIONS TO THE BUILDINGS AND HYDRANTS SHALL BE TESTED IN ACCORDANCE WITH THE TOWN'S WATERMAIN COMMISSIONING PROTOCOL. CONTRACTOR TO PROVIDE DETAILED WRITTEN WATERMAIN COMMISSIONING PROTOCOL FOR APPROVAL PRIOR TO COMMENCING TESTING OPERATIONS. CONNECTIONS TO EXISTING MAINS SHALL NOT BE MADE UNTIL WRITTEN AUTHORIZATION IS PROVIDED BY THE ENGINEER AND THE TOWN.
- FOLLOWING TESTING, THE CONTRACTOR SHALL ARRANGE FOR OPERATION OF EACH SERVICE TO VERIFY FULL FLOW AND PRESSURE AT CURB STOP TO SATISFACTION OF THE ENGINEER.
- EXISTING WATER SERVICES THAT ARE TO BE ABANDONED MUST BE CAPPED AT MAIN INCLUDING MAIN STOP REMOVED, PLUG INSERTED TO TOWN STANDARDS AND REINSTATEMENT OF ALL DISTURBED AREAS TO EXISTING CONDITION OR BETTER.
- WATER VALVES TO BE OPERATED BY TOWN STAFF ONLY.
- ALL WATER USED IN COMMISSIONING AND TESTING TO BE POTABLE AND SUPPLIED FROM THE TOWN OF THE BLUE MOUNTAINS, THE TOWN OF COLLINGWOOD OR THE MUNICIPALITY OF MEAFORD.

MATERIALS

- ALL MATERIAL TO COMPLY WITH CSA, OPSS AND TOWN STANDARDS.
- SANITARY SEWER – PVC DR 35.
- SANITARY SERVICE CONNECTIONS – PVC SDR 28.
- STORM SEWER – PVC SDR 35 OR CONCRETE 65-D. ALL SEWERS TO BE JOINED WITH A GASKETED BALL AND SPIGOT SYSTEM. NON-REINFORCED CONCRETE PIPE MAY BE USED FOR SIZES UP TO AND INCLUDING 375 mm DIA. REINFORCED CONCRETE PIPE MAY BE USED FOR ALL SIZES OF SEWER.
- CULVERTS – GALVANIZED CORRUGATED STEEL PIPE. MIN. WALL THICKNESS 2.0 mm UNLESS OTHERWISE NOTED. ALL CULVERTS TO HAVE END PROTECTION TO OPSS 810.010 (BOTH ENDS), TYPE B – COMPLETE WITH FILTER FABRIC.
- PERFORATED SUBDRAIN – BIG 'O' WITH GEOTEXTILE FILTER SOCK OR APPROVED EQUAL.
- ALL CHEMICALS AND MATERIALS USED IN THE ALTERATION OR OPERATION OF THE DRINKING WATER SYSTEM THAT COME IN CONTACT WITH WATER WITHIN THE SYSTEM SHALL MEET ALL APPLICABLE STANDARDS SET BY BOTH THE AMERICAN WATER WORKS ASSOCIATION ("AWWA") AND THE AMERICAN NATIONAL STANDARDS INSTITUTE ("ANSI") SAFETY CRITERIA STANDARDS NSF/60, NSF/61 AND NSF372.
- WATERMAIN – PVC DR 18.
- WATER SERVICE CONNECTIONS TO BE TYPE 'K' COPPER PIPE, REHAU'S MUNICIPLEX (BLUE).
- WATER SERVICE FITTINGS SHALL BE:
 MAIN STOP – MUELLER H25008
 CURB STOP – MUELLER H25209
 SERVICE SADDLE – ROBAR 2616
 TAPPING SADDLE – ROBAR 6906
 SERVICE BOXES – MUELLER A-726, STAINLESS STEEL RODS
- PIPE RESTRAINTERS – SIGMA C-900.
- HYDRANTS – CLOW D51 CONCORD PREMIER, OPEN LEFT, WITH STORTZ CONNECTIONS ON ALL STEAMER PORTS. HYDRANT SETS SHALL BE INSTALLED NOT LESS THAN 0.9 m FROM THE CENTER OF THE VALVE TO THE CENTER OF THE HYDRANT. TRACER WIRE SHALL BE ATTACHED TO THE OUTSIDE OF THE VALVE BOX AND WIRE BROUGHT INTO VALVE BOX UNDER CAP.
- LIVE TAP SADDLES TO BE EPOXY COATED COMPLETE WITH STAINLESS STEEL BOLTS.
- MECHANICAL JOINT DUCTILE FITTINGS TO AWWA/ANSI C153/A21.53. INCLUDING PROTECTO-CAPS, CAT NO. 175P190 OR APPROVED EQUAL.
- ISOLATION VALVES TO BE RESILIENT SEAT GATE VALVES WITH MECHANICAL JOINTS, OPEN LEFT, CLOW. VALVE BOXES TO BE 5-SL-48 SLIDING OR MUELLER MVB COMPOSITE COMPLETE WITH GUIDE PLATE AND DUCTILE ADJUSTABLE TOP AND LID OR APPROVED EQUAL. CAPS TO BE PAINTED BLUE.
- ALL WATERMAIN FASTENERS (STAINLESS BOLTS) TO BE COMPLETE WITH ZINC CAPS.
- ALL SPECIFIED AGGREGATES TO OPSS 1010.
- RIPRAP TO OPSS 1004.05.05, ON FILTER FABRIC.
- FILTER FABRIC – TERRAFIX 270R OR APPROVED EQUAL.
- INSULATION – STYROFOAM HIGLOAD 40 EXTRUDED POLYSTYRENE FOAM INSULATION, 50mm THICK SHEETS.

WTM/FITTING	ANODE SIZE (kg)	SPACING
100 mm	2.3	PER LENGTH OVER 3.0 m OR FITTING OF SIMILAR SIZE
150 mm	2.3	PER LENGTH OVER 3.0 m OR FITTING OF SIMILAR SIZE
200 mm	5.5	PER LENGTH OVER 3.0 m OR FITTING OF SIMILAR SIZE
250 mm	5.5	PER LENGTH OVER 3.0 m OR FITTING OF SIMILAR SIZE
300 mm	11	PER LENGTH OVER 3.0 m OR FITTING OF SIMILAR SIZE
COPPER SERVICE	2.3	AT EACH CURBSTOP
HYDRANT	5.5	AT HYDRANT BASE
VALVE	5.5	AT EACH VALVE
SERVICE SADDLE	2.3	AT EACH SERVICE SADDLE (CONNECT WIRE TO BOLT ON SADDLE TIGHT BETWEEN 2 NUTS)
TEES, ELBOWS ETC.	2.3	ON EACH FITTING FOR 100 mm – 150 mm PIPE
TEES, ELBOWS ETC.	5.5	ON EACH FITTING FOR 200 mm – 300 mm PIPE

ANODE SIZING CHART FOR ZINC ANODES

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BENCHMARKS

BM1 – ELEVATION 186.52
 ELEVATIONS SHOWN HEREON ARE GEODETIC AND REFER TO THE TOP OF IRON BAR, NORTH WEST CORNER OF LOT.

BM2 – ELEVATION 185.37
 ELEVATIONS SHOWN HEREON ARE GEODETIC AND REFER TO THE TOP OF IRON BAR, NORTH EAST CORNER OF LOT.

BM3 – ELEVATION 188.63
 ELEVATIONS SHOWN HEREON ARE GEODETIC AND REFER TO THE TOP OF IRON BAR, SOUTH WEST CORNER OF WELLINGTON STREET ROW.

NOTES

LEGAL SURVEY INFORMATION AND LOT DIMENSIONS SHOWN ON THIS PLAN ARE TAKEN FROM A SURVEY PLAN PREPARED BY JO BARNES LTD DATED FEBRUARY 25, 2022, WHICH MAY NOT BE FINAL AND ARE NOT GUARANTEED. THE FINAL REGISTERED PLAN SHALL BE REFERRED TO FOR CONFIRMATION OF THE DATA.

TOPOGRAPHIC INFORMATION SHOWN ON THIS PLAN IS FROM A SURVEY PREPARED BY JOETOPO SURVEYS AND CADD INC, DATED MARCH 28, 2020.

No.	REVISION DESCRIPTION	DATE	ENGINEER STAMP



150/160 KING STREET
THORNBURY
TOWN OF THE BLUE MOUNTAINS



DETAILS & NOTES

DESIGN: RD	FILE: 122030	DWG:
DRAWN: RD	DATE: APR 2022	
CHECK: RS	SCALE: NTS	

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