### TRAFFIC IMPACT STUDY

ALTA RESIDENTIAL DEVELOPMENT – PHASE 2

TABERA LTD.

### PREPARED BY:

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**JULY 2007** 

CFCA FILE No. 119-2528

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### INTRODUCTION

CF Crozier & Associates Inc. was retained by Tabera Ltd. to undertake a Traffic Impact Study for Phase 2 of the proposed Alta residential development in the Town of The Blue Mountains, County of Grey. The development is bounded by Hidden Lake Road to the north, the termini of Alta Road to the south and the Niagara Escarpment to the west. The property proposed for residential development is 29.8 hectares. **Figure 1** illustrates the site location.

The development is proposed to consist of 64 single family detached residential units. Access to the site will be provided by extending the existing Alta Road to an intersection with Hidden Lake Road. The internal roadways will be publicly owned as part of the subdivision tenure of the development. **Figure 2** illustrates the proposed layout of the site.

The study has been completed in accordance with the procedures set out in the Institute of Transportation Engineers (ITE) "Transportation Impact Analyses for Site Development" 2005 guide, with the associated analysis and findings outlined herein.

### 1.0 Existing Conditions

### 1.1 BOUNDARY ROAD NETWORK

Hidden Lake Road is a local two lane unpaved road under the jurisdiction of the Town of The Blue Mountains. The speed limit is not posted, therefore by regulation is 50km/h. Hidden Lake Road terminates to the west of the subject development.

Alta Road is a local two lane paved road that is publicly owned by the Town of the Blue Mountains. The segment of Alta Road from Arrowhead Road to approximately 300 metres west has not to date been assumed by the municipality. The speed limit is not posted, therefore by regulation is 50 km/h. Currently, Alta Road terminates in a cul-de-sac at the southern limits of the subject development.

Arrowhead Road is a local two lane paved road under the jurisdiction of the Town of The Blue Mountains. The posted speed limit is 50 km/h.

Highway 26 is a two lane east-west highway under the jurisdiction of the Ontario Ministry of Transportation. The posted speed limit is 80 km/h.

The intersection of Highway 26 and Hidden Lake Road is unsignalized. The south approach (Hidden Lake Road) is stop-controlled with no exclusive turn lanes. Insufficient pavement width exists on the south approach to allow simultaneous left and right turns. The east and west approaches (Highway 26) have no restrictions to free flow with no exclusive turn lanes. No north approach exists at the intersection.

The intersection of Highway 26 and Arrowhead Road is unsignalized. The south approach (Arrowhead Road) is stop-controlled with no exclusive turn lanes. Sufficient pavement width exists on the south approach to allow simultaneous left and right turns. The east and west approaches

(Highway 26) have no restrictions to free flow with no exclusive turn lanes. No north approach exists at the intersection.

The intersection of Arrowhead Road and Alta Road is unsignalized. The west approach (Alta Road) is stop-controlled with no exclusive turn lanes. Insufficient pavement width exists to allow simultaneous left and right turns. The north and south approaches (Arrowhead Road) have no restrictions to free flow as well as no exclusive turn lanes. No east approach exists at the intersection.

### 1.2 TRAFFIC DATA

Turning movement counts were undertaken by CF Crozier & Associates staff from 7:00 to 9:00 a.m. and 4:00 to 6:00 p.m. on Friday June 15, 2007 at the intersections of Highway 26 and Arrowhead Road, Highway 26 and Hidden Lake Road and Arrowhead Road and Alta Road. This traffic data provides the typical roadway a.m. and p.m. peak traffic conditions through the above mentioned intersections, and was selected to capture both commuter and weekend recreational traffic from the GTA region. At the intersections Highway 26 with Arrowhead Road and Hidden Lake Road, the a.m. peak hour was 7:30 to 8:30 a.m., and the p.m. peak hour from 4:45 to 5:45 p.m. At the intersection of Arrowhead Road and Alta Road the a.m. peak hour was 8:00 to 9:00 a.m. and the p.m. peak hour was 4:00 to 5:00 p.m. The traffic data contained in **Appendix B** provides a summary of the turning movement counts. **Figure 3** illustrates the existing traffic volumes.

### 1.3 INTERSECTION OPERATIONS

The operations of subject intersections were analyzed on the basis of the traffic volumes illustrated in Figure 3. Detailed capacity analyses are included in **Appendix C**.

The assessment of unsignalized intersections is based on the method outlined in the "Highway Capacity Manual, 2000". The Level of Service definitions for unsignalized intersections are included in **Appendix A**.

Table 1 outlines the current levels of service.

TABLE	<b>5</b> 1
2007 Existing Traffic	LEVELS OF SERVICE

Approach	Peak Hour	Level of Service	Control Delay	95%ile Queue Length	Volume-to- Capacity
Highway 26 and	A.M.	В	11.1 s	0.05	0.02
Arrowhead Road	P.M.	В	14.6 s	0.07	0.02
Highway 26 and Hidden	A.M.	В	10.7 s	0.01	0.00
Lake Road	P.M.	С	17.2 s	0.00	0.00
Arrowhead Road and	A.M.	Α	8.7 s	0.05	0.02
Alta Road	P.M.	А	8.6 s	0.06	0.02

Note: The level of service of a stop-controlled intersection is based on the delay associated with the critical minor road movement.

As indicated in Table 1, all intersections operate at acceptable levels of service and are characterized by low volume-to-capacity ratios and 95% queue lengths of one vehicle.

### 3.0 FUTURE BACKGROUND CONDITIONS

### 3.1 HORIZON YEAR

As per the ITE "Transportation Impact Analyses for Site Development" manual, the horizon year was chosen to be the anticipated full build out year. With an assumed construction of the internal road network and services in 2008, it was determined that a horizon year of 2014 would capture full build out of the development and was used for analysis.

### 3.2 TRAFFIC GROWTH RATES

Traffic growth rates were based on Georgian Triangle Area Transportation Study (Dillon Consulting Ltd. & Ainley and Associated Ltd., June 2001). This report was a comprehensive examination of the future transportation needs of the Georgian Triangle driven by growth in population and tourism. The report forecasted ten and twenty year demands on major routes in the area, including Highway 26.

The report provided traffic volume forecasts for Highway 26 at Arrowhead Road. Growth rates were calculated from these forecasts, and were applied to Highway 26 through volumes at both the Arrowhead Road and Hidden Lake Road intersections. A compound growth rate of 8.2 percent to 2010 and 2.1 percent thereafter was used for Highway 26 eastbound. A compound growth rate of 8.3 percent to 2010 and 2.1 percent thereafter was used for Highway 26 westbound.

### 3.3 OTHER LOCAL DEVELOPMENTS

Two other developments that will directly influence future background traffic volumes are currently either under construction or in the planning phase. These are Georgian Woodlands Phase 4 and Alpine Springs.

The Georgian Woodlands Phase 4 development is seeking draft plan approval for 102 single family detached units, and is located east of the subject property, east of Arrowhead Road. One portion of the development will have a singular access onto Arrowhead Road north of Alta Road and will service 36 units. Four units are proposed to access Arrowhead Road directly, south of Alta Road. The remaining 62 units will access Arrowhead Road north of Alta Road, or other boundary roadways to the southeast of Alta Phase 2 development. Trip generation and distribution characteristics as per Section 4.0 were applied to the 102 units.

Alpine Springs will consist of 15 single family detached units and 15 single family attached units located south of the subject development. It has a singular access to the Arrowhead Road south of Alta Road. Trip generation and distribution characteristics as per Section 4.0 were applied to these 30 units.

The calculated 2014 future background traffic volumes are illustrated in **Figure 4** and reflect the growth on Highway 26 as well as the traffic generated from the two nearby future developments.

### 3.4 INTERSECTION OPERATIONS

The operations of the critical intersections were analyzed on the basis of the traffic volumes illustrated in Figure 4. **Table 2** outlines the year 2014 levels of service. Detailed capacity analyses are included in Appendix C.

Table 2
2014 Future Background Traffic Levels of Service

Approach	Peak Hour	Level of Service	Control Delay	95%ile Queue Length	Volume-to- Capacity
Highway 26 and	A.M.	В	12.2 s	0.10	0.03
Arrowhead Road	P.M.	С	17.9 s	0.20	0.06
Highway 26 and Hidden	A.M.	В	12.0 s	0.02	0.01
Lake Road	P.M.	С	24.9 s	( <del>ee</del>	
Arrowhead Road and	A.M.	Α	8.8 s	0.05	0.02
Alta Road	P.M.	Α	8.7 s	0.06	0.02

Note: The level of service of a stop-controlled intersection is based on the delay associated with the critical minor road movement.

As can be seen in Table 2, the critical intersections under future background traffic volumes will continue to operate at an acceptable level of service, maintaining low volume-to-capacity ratios, slight increases in delays and 95% queue lengths of one vehicle.

### 4.0 SITE GENERATED TRAFFIC

### 4.1 TRIP GENERATION

The Georgian Triangle Area is branded as a four seasons recreational destination. As a result, many new developments in the Town of The Blue Mountains market to and attract those who have their primary residence elsewhere, mainly the Greater Toronto Area. Accordingly, trip generation rates of existing developments are frequently less than predicted by the Institute of Transportation Engineers (ITE) "Trip Generation Manual", which are reflective of residential urban and suburban areas.

This was supported by the turning movement count undertaken at Arrowhead Road and Alta Road as well as Highway 26 and Hidden Lake Road. It was observed that a significant percentage of overall entering and exiting traffic into the Alta Phase 1 development were for the construction of two residences. At Hidden Lake Road, the 62 units serviced by the roadway generated a total of 7 trips during each of the a.m. and p.m. peak hours, equating to a trip generation rate of 0.11 trips per unit.

It was determined that the use of Category 260, "Recreational Housing" would be an accurate representation of the future traffic characteristics of the Alta Phase 2 development and was therefore used to model the 64 units in the analysis. The trips generated are tabulated in **Table 3**.

TABLE 3
SITE GENERATED TRIPS

	Roadway Peak	Nu	ımber of Trips	
ITE Land Use	Hour	Inbound	Outbound	Total
	A.M.	10	10	20
Category 260	P.M.	9	11	20

### 4.2 TRIP DISTRIBUTION AND ASSIGNMENT

The trips generated by the proposed residential development were distributed to the boundary roadways based on the location of retail, commercial and recreational destinations. With the Town of Collingwood and the Village at Blue both located to the east of the subject lands, 80 percent of trips were assumed to arrive from/depart to the east. Specifically, 50 percent would access Highway 26 east from Hidden Lake Road and 30 percent would access Arrowhead Road south from Alta Road. The remaining 20 percent were assigned to Highway 26 west from Hidden Lake Road. The trip distribution is illustrated in **Figure 5**.

The trips generated by the proposed development were assigned to the boundary road network as per the distribution. **Figure 6** illustrates the trip assignment.

### 5.0 TOTAL FUTURE CONDITIONS

### 5.1 BASIS OF ASSESSMENT

The traffic impacts arising from the proposed development were assessed on the basis of the site generated traffic illustrated in Figure 6 being superimposed on the future background traffic volumes in Figure 4. The resulting total traffic volumes for the a.m. and p.m. peak hours are illustrated in **Figure 7**.

### 5.2 INTERSECTION OPERATIONS

The intersection levels of service were analyzed on the basis of the total traffic volumes illustrated in Figure 7. **Table 4** outlines the year 2014 total traffic levels of service. Detailed capacity analyses are included in Appendix C.

TABLE 4
2014 TOTAL TRAFFIC LEVELS OF SERVICE

Approach	Peak Hour	Level of Service	Control Delay	95%ile Queue	Volume-to- Capacity
Highway 26 and	A.M.	В	12.2 s	Length 0.11	0.03
Highway 26 and Arrowhead Road	P.M.	С	17.8 s	0.20	0.06
Highway 26 and	A.M.	В	12.1	0.03	0.01
Arrowhead Road	P.M.	С	17.7 s	0.05	0.02
Arrowhead Road and	A.M.	Α	8.9 s	0.07	0.02
Alta Road	P.M.	А	8.7 s	0.07	0.02

Note: The level of service of a stop-controlled intersection is based on the delay associated with the critical minor road movement.

As indicated in Table 4, the critical intersections will continue to operate at acceptable levels of service, with insignificant increases in delay, 95<sup>th</sup> percentile queue lengths of one vehicle and low volume-to-capacity ratios.

The intersection of Highway 26 and Hidden Lake Road will see decrease in control delay from the future background traffic operations. This decrease in delay, although having introduced greater

traffic volumes, is a result of more northbound right-turns thereby reducing the average delay of all northbound vehicles.

### 5.3 AFFECT ON LOCAL ROADS

An assessment of the affect of the proposed development on local roads was undertaken. Category 260, "Recreational Housing" of ITE Trip Generation Manual forecasts an average of 3 trips per unit per weekend day. With a total of 64 units, this equates to 195 new trips on Alta Road and Hidden Lake Road. Assuming that that these trips occur over a 12 hour day, 16 trips per hour can be expected on Alta Road and Hidden Lake Road. Using the trip distribution from Figure 5, both Alta Road and Hidden Lake Road will experience an average of eight additional inbound and outbound trips per hour on a weekend.

### 6.0 CONCLUSIONS

Intersection analyses of existing traffic volumes indicate that the critical intersections are currently operating at acceptable levels of service. Intersection analyses of the 2014 future background traffic volumes indicate that the critical intersections are expected to continue operating at acceptable levels of service with minor increases in delay.

The proposed residential development is expected to add 20 new trips to the boundary road system during both the a.m. and p.m. peak hours. The majority of these new trips are expected to arrive from/depart to the east. Intersection analyses of the 2014 total traffic volumes indicate that the critical intersections are expected to continue to operate at acceptable levels of service and that the addition of the new trips will not materially affect the operations of the intersections in either the a.m. or p.m. peak hours.

The proposed Alta Phase 2 development is expected to add an average of eight vehicles per hour throughout a weekend day on both Hidden Lake Road and Alta Road. These additional vehicles will not significantly affect the operations of these local roadways.

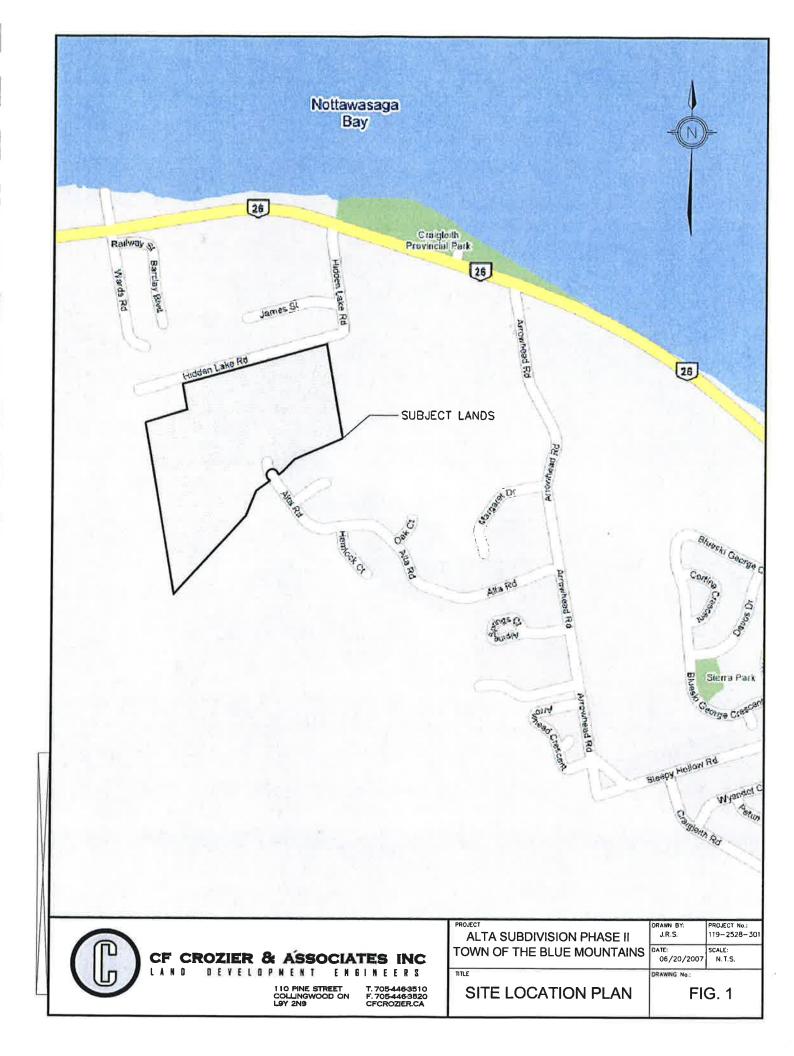
It is concluded that the traffic generated from the proposed residential development will not adversely affect the boundary road system and that no external improvements are required.

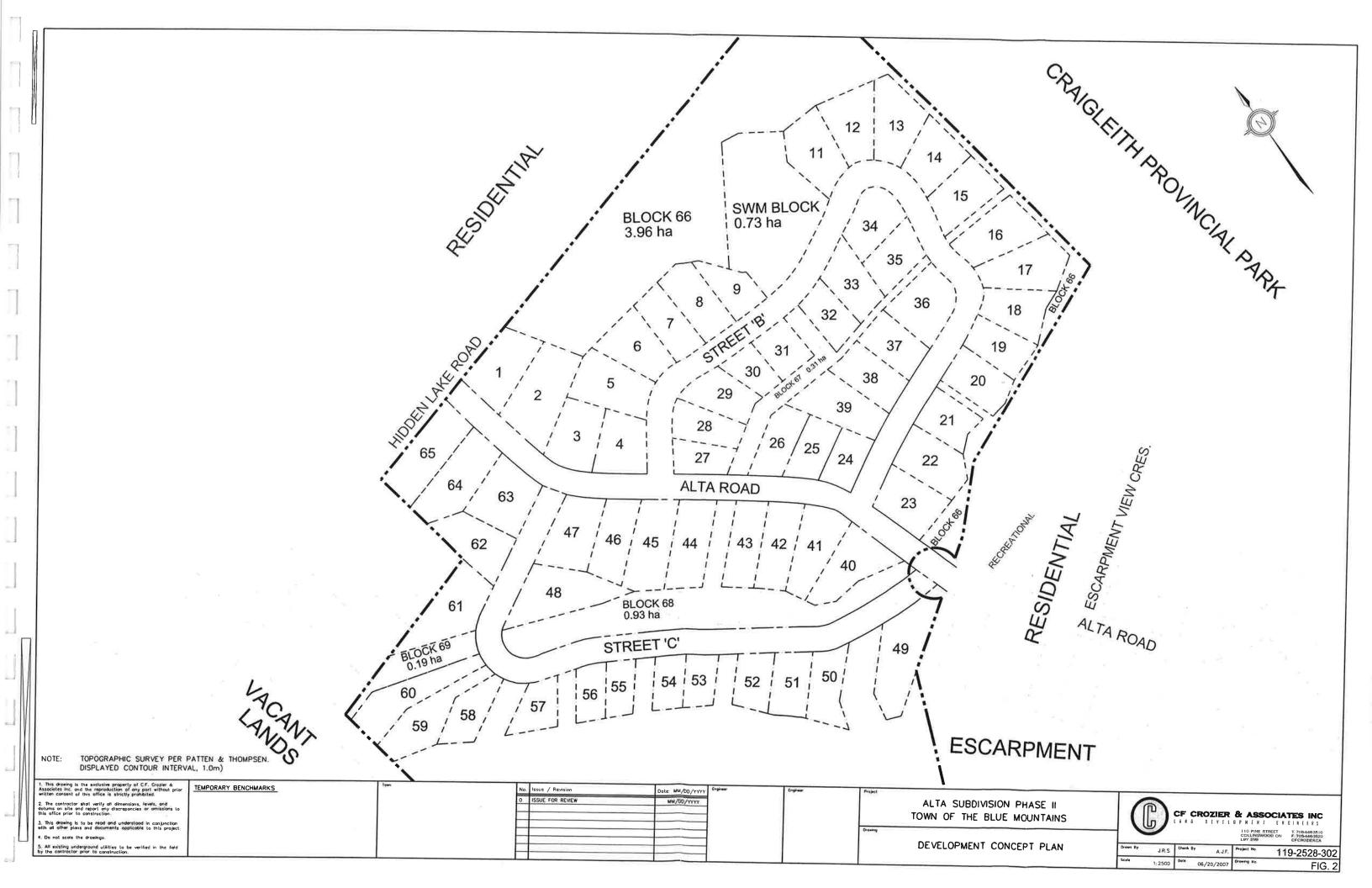
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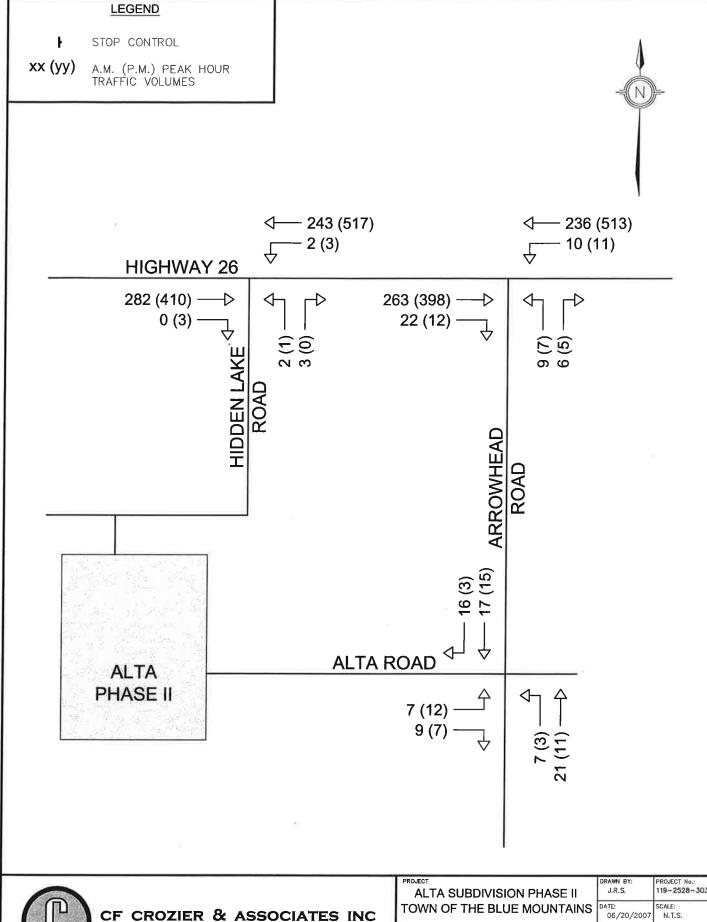
C.F. CROZIER & ASSOCIATES INC.

Alexander J. W. Fleming, MBA, P.Eng.

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DEVELOPMENT ENGINEERS

110 PINE STREET COLLINGWOOD ON L9Y 2N9

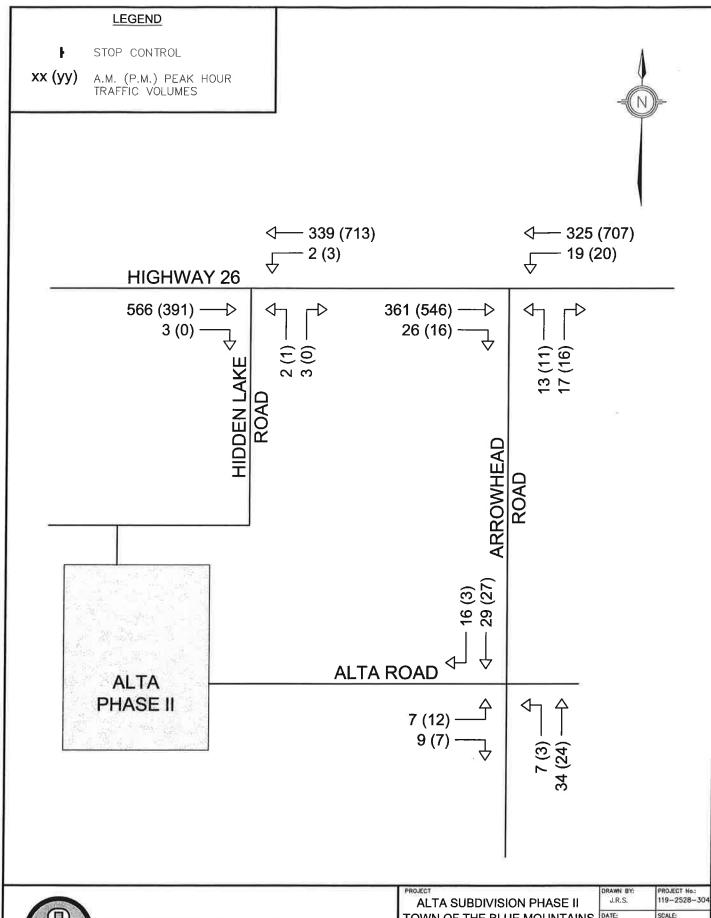
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**EXISTING** TRAFFIC VOLUMES

FIG. 3



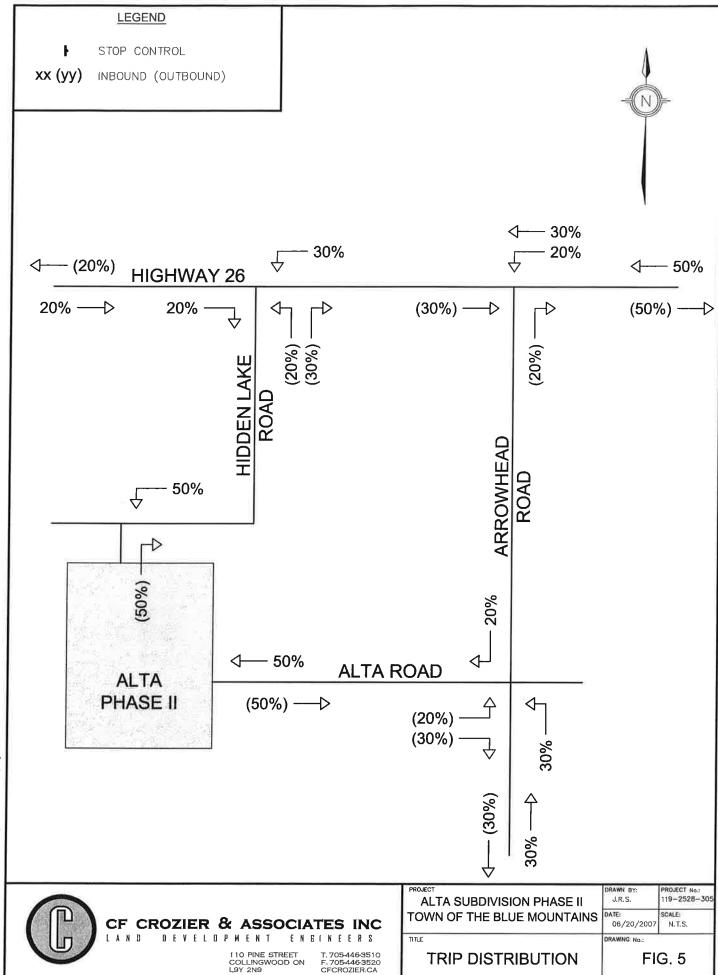
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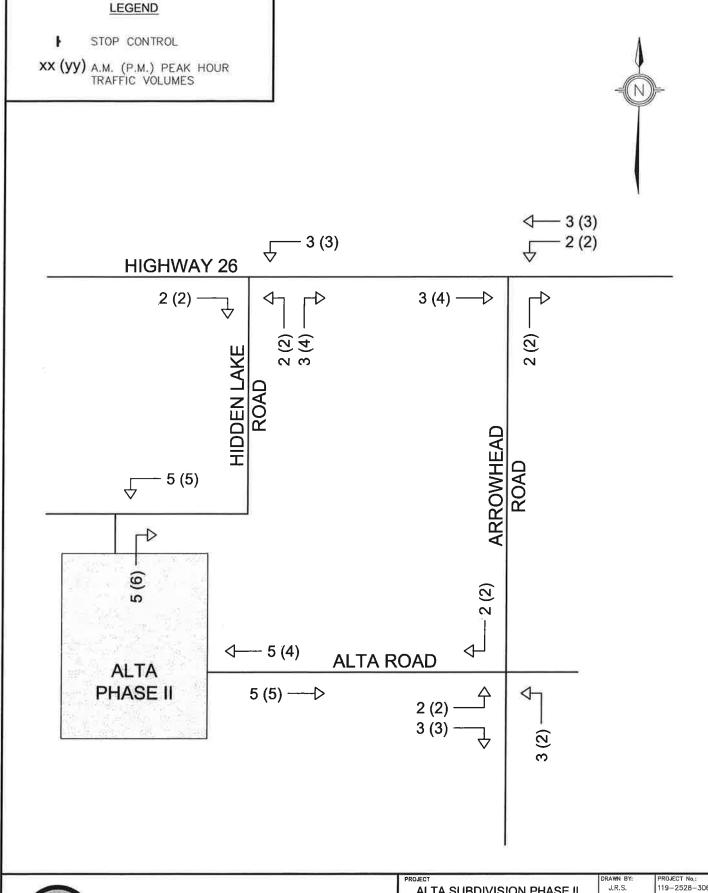
2014 FUTURE **BACKGROUND TRAFFIC VOLUMES** 

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FIG. 4



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CF CROZIER & ASSOCIATES INC

110 PINE STREET COLLINGWOOD ON L9Y 2N9 T. 705-446-3510 F. 705-446-3520 CFCROZIER.CA ALTA SUBDIVISION PHASE II
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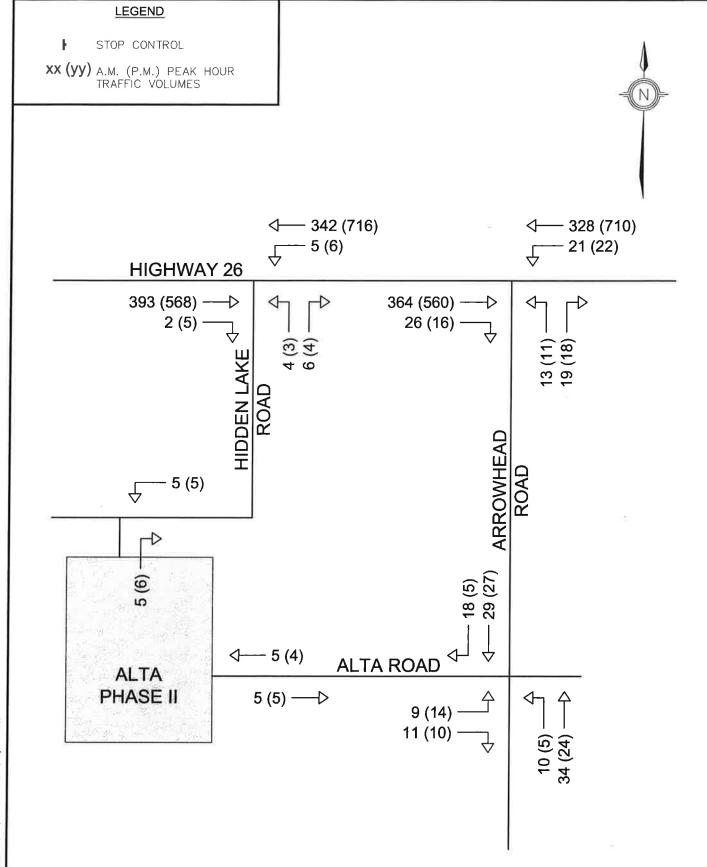
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TRIP ASSIGNMENT TRAFFIC VOLUMES

FIG. 6

SCALE:

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CF CROZIER & ASSOCIATES INC

DEVELOPMENT

110 PINE STREET COLLINGWOOD ON L9Y 2N9

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ALTA SUBDIVISION PHASE II TOWN OF THE BLUE MOUNTAINS

J.R.S. DATE

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2014 TOTAL FUTURE TRAFFIC VOLUMES

FIG. 7

# APPENDIX A

Level of Service Definitions

### **LEVEL OF SERVICE DEFINITIONS**

### Two-Way Stop Controlled Intersections

Level of Service	Control Delay per Vehicle (seconds)	Interpretation
А	≤ 10	EXCELLENT. Large and frequent gaps in traffic on the main roadway.  Queuing on the minor street is rare.
В	> 10 and ≤ 15	VERY GOOD. Many gaps exist in traffic on the main roadway. Queuing on the minor street is minimal.
С	> 15 and ≤ 25	GOOD. Fewer gaps exist in traffic on the main roadway. Delay on minor approach becomes more noticeable.
D	> 25 and ≤ 35	FAIR. Infrequent and shorter gaps in traffic on the main roadway. Queue lengths develop on the minor street.
E	> 35 and ≤ 50	POOR. Very infrequent gaps in traffic on the main roadway. Queue lengths become noticeable.
F	> 50	FAILURE. Very few gaps in traffic on the main roadway. Excessive delay with significant queue lengths on the minor street.

Source: Highway Capacity Manual 2000, Transportation Research Board

# APPENDIX B

**TURNING MOVEMENT COUNTS** 

Job Number:

119-2528 15-Jun-07 Ben Prashaw Date: Counter:

North Leg Road Name N/A South Leg Road Name Hidden Lake Road East Leg Road Name: Hwy 26 West Leg Road Name Hwy 26

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Job Numbe 119-2528 Date: 15-Jun-07

Date: 15-Jun-07 Counter: Jeremy Ash

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513

Job Numbt 119-2528
Date: 15-Jun-07
Counter: Torben Ruddock

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O		ļ.	,	L	L	L	ŀ	L	·	_	ALL RED START	DST	<b>ART</b>			_	EV CLR	α,	S	COND SERVICE					ပ	INITAL		1	
Ω		-1	Ŀ	L		L	'	L	٠	_	(F/1 + C + 0) =	9+0	=	5.0	_		EV DLY	, Y	D	MAN CONT CALL			$\exists$			< C + O + F	<u> </u>		
Ш		2.0	5.9		L	L	5.9	<u></u>	5.0	1=	RED REVERT	EVEF	Ţ		ı		RR CLR	œ,	Е	YELLOW START	Ě		$\stackrel{\times}{\Box}$		<del></del> i				
Ш	L"	,	1.5		_	L	1.5	LC	1.4		(F/1 + O + F) =	J + C	= (	5.0	_		RR DLY	Υ.	F	FIRST PHASES			$\exists$		<u>Ş</u> ī	ONTARIO 233 PROGRAM	233 PF	ROGRAM	
J	Īσ	镁		ပ V	0+	4+	11	<b> </b>		1					ı						S V	0	Ⅱ	<u>, , , , , , , , , , , , , , , , , , , </u>		HWY 26	at RR 1	HWY 26 at RR 19/BLUE MTN RD	TN RD.
ļ		-								İ		i		-					+-						-		-		
		J	Column E Phases / Bits	II E	Pha	ses	/ Bits	"				_	Column F	Jn F	Pha	Phases / Bits	Bits						띩	1		Dial-out Telephone		REDIAL TIME - 10	ME - 10
		Ŀ	Ļ	1	4	5	9		00	_			2	2	$\overline{}$	2	_	7 8			1 2	3	4 5 6	1	8	Number		(C/2+C+0)	(0+
9	BXCITISIVE	1	+	┿	+	┰	┰	╀	╁	<u> </u>		-	-	⊢	╀			$\vdash$	0	ADV GRN FLH	×		E	F	0	NO, OF DIGITS	11	DEFAULT IS : REPORT	: REPORT
1	┸	+	1	1	1	1	+	+	1	1	EXT PERMIT	1	-	L	L			-	<u> </u>	PHASE FLASH					-	1st DIGIT	1	ALL ALARMS	6
- -		+	+	1	+	1	+	+	+	- -		12	+	-	L			+	2	FLASH WALK	F		L		2	2nd DIGIT	<u></u> б	(NO FLAGS SET)	SET)
4 0		+	-	+	+	+	+	+	+	1 6		<u></u>	+	-	L			$\vdash$	<u>س</u>	GUAR PASS				F	6	3rd DIGIT	0		
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4 r	_	+	1	1	+	+	+	+	+	· u	PED 2P OLIT	1	ľ	+	1			-	5	┺		F	F	F	5	5th DIGIT	7 ALARM		Column F
٥١	_	+	+	4	+	+	+	+	+	10	-	+	+	1	1			+	l «	_	‡		t	F	9	6th DIGIT	O REP	REPORTING 1	2345678
ဖ	_	-	4	4	4	4	+	+	+	1 0	_	1	+	+	1		1	+	1	L	‡	+	‡	#	  -	7th DIGIT	_	1	×
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∞	DISABL OVP YEL	_	-	4	4	4	+	+	4	∞		<del> </del>	+	4	1		1	4	#	EXI RECALL		+	#	‡		Figure 48		1 = STOP TIME	
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ပ	EM VEH C	Н		4	$\perp$	_		4	_	이		+	-	4	4		1	+	기	SEMI ACTUATD		+		#			1		4 - MANOAL FLAMA 6 - CNAPI E BOLICE CONTROL
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ш	L	×	L	×	_	×	L	-	_	쁘	E RESTRICTED	9							Ш	E STRT VEH CALL	×	$\exists$		$\exists$	Ш		] 	6 = EXTERNAL ALARM	ALARM
Ш	L	+	×	+	+	1	+	-	_	1"	F EXTRA 2	2	-						<u> </u>	= STRT PED CALL		×		×	×		7=1	7 = DETECTOR FAILURE	FAILURE
1	_	-	-	1	-	-	1	{`	0+0>		1.11	1	-	1					<u> </u>	SPECIAL	\LS <	ပ	+0+F	=2>	V	C+0+C	=5 >		
	EI ACU TO DEFEMBL	200	TOM:						)		EXTRA 1								!			EX	<b>EXTRA 2</b>					IC SELECT	
	) HSA17	1		_	7	Jar.	1 - TBC TVBE 1	7 7		í	5 = EXPANDED STATUS REPORTING	ON V	[S. Cl	TATIL	SRFF	PORT	Ü			1 = AWR ON DURING PHASE INITAL	N DUF	ZING F	HAS	FINIT	٦Ł	2 = 2  WAY MODEM	MODEM	5 = SIN	5 = SIMPLES MASTER
	Z A L L	. נ		_ ^	- c	1	: Ú		2 - NEMA EXT COODD	_	6 = INTERNATIONAL PED	PRNA	NO	4						2 = LMU INSTALLED	STALL	Œ.				3 = 7 WIRE SLAVE	SLAVE	7 = 7 W	7 = 7 WIRE MASTER
	2 = EVB	0 1	0 = KKZ 7 = SE1	NI -	7 K	֓֞֞֝֟֝֟֝֓֓֓֟֝֟֓֓֓֓֓֓֓֓֓֓֓֓֟֟֓֓֓֓֓֓֓֓֓֓֟֓֓֓֓֓֓	1 6	7. Ž	2 = NEMA EXT. COORD. 3 = DAYLIGHT SAVINGS	; ;;	7 = CI FAR OUTPUTS DURING FLASH	AR C	JATO JATO	JEST	) URIN	IG FL	ASH									4 = FLASH / FREE	/ FREE	8 = 0FI	8 = OFFSET INTURP
	) (i	- 0	֝֝֞֝֟֝֝֝֟֝֝֟֝֓֓֓֓֓֓֓֓֓֓֓֓֡֝֡֓֓֓֡֓֜֝֓֡֓֜֝֡֓֡֓֡֡֡֡֡֓֜֝֡֡֡֡֡֡֡֡			5	5	<u> </u>		)	S - SDI IT DING	, <u>a</u>																	
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PRETIMED ONTARIO 233 PROGRAM

T.O.D. FUNCTIONS

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		^						16						$\perp$	$\perp$	$\perp$		Ц	
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		1,	PERMIT	RED LOCK	YELLOW LOCK	VEH MIN CALL	PED RECALL	PEDESTRIANS	REST IN WALK	RED REST	DOUBLE ENTRY	VEH MAX CALL	SOFT RECALL	MAXIMUM 2	CORD SERVICE	MAN CONT CALL	YELLOW START	FIRST PHASES	<c+o+f=1></c+o+f=1>
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		4	35	5	(4	<u>)                                     </u>	•	5.0	5.0	5.0	20		Ŀ	i,		,	5.9	1.5	< C + O + F = 1 >
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	PHASE	4																	+ 0
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		-	ᠰ	1.	5	1	ī	2.0 5.0	2.0 5.0	2.0 5.0	10)50	١.	•	1	ı	•	2.0	•	Ϋ́
			NAM K	DON'T WALK	MIN INTIAL	TYPE 3 LIMIT	ADD PER VEH		П	MIN GAP	MAX LIMIT	MAXIMUM 2	ADV / DLY WALK	SEQUENCE TO	COND SRV MIN	REDUCE EVERY		RED CLEAR	PHASE BANK#
					Ē	F	AD	>	Ž	Σ	L₹	Į₹	A P C	SEO	S S	RED		A.	] _

# LOCATION: HWY 26 & GREY RD 19 (BLUE MTN.RD.)

Issued Date: MA			Installed Date:		
BI Tran Systems, Inc.	510 Bercut Dr., Sacremento, Calif. 95814	916/441-0260	Traffic Signal Program 233 Ontario	Timing Sheet #2	Revised (02/95)

Issued Date: MAR/2002

BIT 4 - DISABLE DET OFF MONITOR BIT 8 - REAL TIME SPLIT MONITOR BIT 7 - DET COUNT MONITOR F = OUTPUT BITS 1 THRU 4 C = CONDITIONAL SERVICE E = BIT 1 - LOCAL OVERIDE A = VEH SOFT RECALL D = LAG PHASES B = MAXIMUM 2 3 = VEH MIN RECALL 0 = PERMIT PHASES 9 = VEH MAX RECALL 8 = DOUBLE ENTRY 2 = YELLOW LOCK 6 - REST IN WALK 4 = PED RECALL 7 = RED REST 1 = RED LOCK

T.O.D. FUNCTIONS

# APPENDIX C DETAILED CAPACITY ANALYSES

		VO-WAY STOP							
General Information			Site Ir	nform	ation				
Analyst	Alex Flem		Interse				Arrowhea		
Agency/Co.		ier & Associates	Jurisdi				Town of 7		
Date Performed	6/18/2007		Analys	is Yea			2007 Exis	ting Volun	nes
Analysis Time Period	A.M. Pea	k Hour							
Project Description 119			h		144. A		and Doord		
East/West Street: Alta R					treet: A (hrs): 0.		ad Road		
ntersection Orientation:			Study F	enod	(ms). U.	20			
Vehicle Volumes an	d Adjustmen						04	ام مد	
Major Street		Northbound	3		ļ .		Southbou 5	ina	6
Movement	1	2 T	R		L		T		R
Volume (veh/h)	7	21	<del>                                     </del>		<u> </u>		17		16
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.0	00	1.00		1.00
Hourly Flow Rate, HFR			+						
veh/h)	7	0	9		0		0		Q
Percent Heavy Vehicles	29	(***)			0		<u></u>		( <del>***</del>
Median Type				Undi	vided				
RT Channelized			0						0
anes	0	1	0		0		1		0
Configuration	LT								TR
Jpstream Signal		0					0		
Minor Street		Eastbound					Westbou	nd	
Movement	7	8	9		1	0	11		12
	L	Т	R		L		T		R
/olume (veh/h)	7		9						
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.0	0	1.00		1.00
Hourly Flow Rate, HFR veh/h)	0	17	16		7		21		0
Percent Heavy Vehicles	0	0	11		0		0		0
Percent Grade (%)		0					0		
Flared Approach		N					N		
Storage		0					0		
RT Channelized			0						0
anes	0	0	0		0		0		0
Configuration		LR							
Delay, Queue Length, ai	nd Level of Serv	ice							
Approach	Northbound	Southbound		Westb	ound			Eastbound	t
Movement	1	4	7	8		9	10	11	12
ane Configuration	LT							LR	
(veh/h)	7							16	
C (m) (veh/h)	1421	i						990	
/c	0.00				$\dashv$			0.02	
		*					<b>-</b>	0.02	-
95% queue length	0.01								
Control Delay (s/veh)	7.5							8.7	
_OS	Α							A	
Approach Delay (s/veh)	i <del></del>							8.7	
Approach LOS								Α	

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	Т	WO-WAY STOR	CONTR	OL SU	MMAR	Υ			
General Information		*	Site Ir	nforma	ation				
Analyst	Alex Flei	ming	Interse	ction			Arrowhea	d Rd. & All	a Rd.
Agency/Co.		zier & Associates	Jurisdi	ction				he Blue M	
Date Performed	6/18/200		Analys	is Year			2007 Exis	ting Volum	ies
Analysis Time Period	P.M. Pea	ak Hour .							
Project Description 119									
East/West Street: Alta R							ad Road		
Intersection Orientation:			Study F	Period (I	hrs): <i>0.</i> .	25			
Vehicle Volumes an	<u>d Adjustmer</u>						O a vitta la avi		
Major Street		Northbound	3				Southbou 5	ina	6
Movement	1	2 T	R		4		T 7		R
Volume (veh/h)	3	11	- K	$\overline{}$			15	_	3
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.0	0	1.00		1.00
Hourly Flow Rate, HFR			1	$\neg \neg$					
veh/h)	12	0	7		0		0		0
Percent Heavy Vehicles	0		-		0				
Median Type				Undiv	rided				
RT Channelized			0						0
anes	0	1	0		0		1		0
Configuration	LT								TR
Jpstream Signal		0					0		
Minor Street		Eastbound						Westbound	
Movement	7	8	9		10		11		12
	L	T	R	$\rightarrow$			Т		R
/olume (veh/h)	12	4.00	7		4.0	^	1.00		1.00
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.0	0	1.00		
Hourly Flow Rate, HFR veh/h)	0	15	3		3		11		0
Percent Heavy Vehicles	0	0	0		0		0		0
Percent Grade (%)		0					0		
Flared Approach		N					N		
Storage		0					0		
RT Channelized			0						0
anes	0	0	0		0		0		0
Configuration		LR				ž			
Delay, Queue Length, ar	nd Level of Ser	vice					249		
Approach	Northbound	Southbound		Westbo	ound			Eastbound	
/lovement	1	4	7	8		9	10	11	12
ane Configuration	LT							LR	
(veh/h)	3							19	
C (m) (veh/h)	1612							1014	
r/c	0.00							0.02	
95% queue length	0.01						ì	0.06	1
Control Delay (s/veh)	7.2				$\neg \vdash$		1	8.6	i —
OS	A						1	A	<b>†</b>
Approach Delay (s/veh)							1	8.6	1
							1	A	
Approach LOS									

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	Τ\	NO-WAY STOP	CONTR	OL SU	MM.	ARY	-			
General Information			Site I	nforma	atio	n				
Analyst	Alex Flen	ning	Interse	ection			Hwy. 26 8	. Hidden	Lake I	Rd.
Agency/Co.	C.F. Croz	ier & Associates	Jurisdi	ction	-		MTO			
Date Performed	6/18/2007	7	Analys	is Year			2007 Exis	ting Volu	ımes	
Analysis Time Period	A.M. Pea	k Hour								
Project Description 119										
East/West Street: Highw						Hidden L	ake Road			
Intersection Orientation:	East-West		Study F	Period (h	nrs):	0.25				
Vehicle Volumes an	d Adjustmen									
Major Street		Eastbound	1 0				Westbou	nd		
Movement	<del></del>	2	3		_	4	5		6 R	
Values a (value)	<del></del>	7 282	R 0			<u>L</u>	243	-+		
Volume (veh/h) Peak-Hour Factor, PHF	1.00	1.00	1.00	_		1.00	1.00		1.00	<del></del>
Hourly Flow Rate, HFR				-			<b>-</b>			
(veh/h)	0	282	0			2	243		0	
Percent Heavy Vehicles	0	-				0	<u> </u>			
Median Type			_	Undiv	ided					
RT Channelized			0						0	
Lanes	0	1	1			0	1		0	
Configuration		T	R			LT				
Upstream Signal		0	1				0			
Minor Street		Northbound					Southbou	ınd		
Movement	7	8	9			10	11		12	
	L	Т	R			L	Т		R	
Volume (veh/h)	2		3			1.00	4.00			
Peak-Hour Factor, PHF	1.00	1.00	1.00			1.00	1.00	<u> </u>	1.00	
Hourly Flow Rate, HFR (veh/h)	2	0	3			0	0		0	
Percent Heavy Vehicles	0	0	0			0	0		0	
Percent Grade (%)		0					0			
Flared Approach		N					T N			
Storage		1					0			
RT Channelized			0						0	
Lanes	0	0	0			0	0		0	
Configuration	Î	LR								
Delay, Queue Length, ar	d Level of Serv	rice								
Approach	Eastbound	Westbound		Northbo	und		5	Southbou	ınd	
Movement	1	4	7	8		9	10	11		12
Lane Configuration		LT		LR						
v (veh/h)		2		5						
C (m) (veh/h)		1292		638						
v/c		0.00		0.01						
95% queue length		0.00		0.02						
Control Delay (s/veh)		7.8		10.7						
LOS		Α		В						
Approach Delay (s/veh)				10.7						
Approach LOS				В						

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		Site In	formation	on			
Alex Flem	ina	Interse	ction		Hwy. 26 8	Hidden L	ake Rd.
		9-29-29-29-29-29-29-29-29-29-29-29-29-29			MTQ		
6/18/2007		Analysi	s Year		2007 Exis	ting Volur	nes
P.M. Peak	Hour						
-2528							
ay 26					_ake Road		
East-West		Study P	eriod (hrs)	: 0.25	- 4000		
Adjustment	s						
1	Eastbound				Westbou	nd	
1	2	3		4	5		6
L							R
							4.00
1.00	1.00	1.00		1.00	1.00		1.00
0	410	3		3	517		0
0							
			Undivide	d			
							0
0					1		0
		R		LT			
		400				ınd	40
7							12
L	Т			L	<u> </u>		R
				4.00	1.00		1.00
1.00	1.00	1.00		1.00	1.00		
1	0	0		0	0		0
		0		0			0
	N						
	1			- 11	0		
		0					0
0	0	0		0	0		0
	LR				<u></u>		
d Level of Serv	ice						
Eastbound	Westbound	ı	Northboun	d	S	Southbour	nd
1	4	7	8	9	10	11	12
	LT		LR				
			1				
					1		
					+		
					-		
					-		
					<del>-</del>		
	Α		C				
			17.2				
	C.F. Crozi 6/18/2007 P.M. Peak 2528 Bay 26 East-West  I Adjustment  1 L 1.00 0 0 7 L 1.00 1 0 0 d Level of Serv Eastbound	Adjustments   Eastbound     1	Alex Fleming   C.F. Crozier & Associates   G/18/2007   P.M. Peak Hour	Alex Fleming	C.F. Crozier & Associates   6/18/2007   P.M. Peak Hour	Alex Fleming	Alex Fleming

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	T	WO-WAY STOR	CONTR	OL SUM	MARY			
General Information			Site I	nformati	on			
Analyst	Alex Fler	nina	Interse	ection		Hwy. 26 8	Arrowhea	d Rd.
Agency/Co.		zier & Associates	Jurisdi			MTO		0.00
Date Performed	6/18/200		Analys	is Year		2007 Exis	ting Volum	es
Analysis Time Period	A.M. Pea	k Hour						
Project Description 119	9-2528							
East/West Street: Highw			North/S	South Stree	et: Arrowhe	ad Road		
Intersection Orientation:	East-West		Study I	Period (hrs	): 0.25			
Vehicle Volumes an	d Adjustmen	its						
Major Street		Eastbound				Westbou	nd	
Movement	1	2	3		4	5		6
	L	Т	R			T		R
Volume (veh/h)		263	22		10	236		4.00
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
Hourly Flow Rate, HFR (veh/h)	0	263	22		10	236		0
Percent Heavy Vehicles	0	<del>2</del>			0			**
Median Type				Undivide	d			
RT Channelized			0					0
Lanes	0	1	1		0	1		0
Configuration		T	R		LT			
Upstream Signal		0				0		
Minor Street		Northbound				Southbou	ınd	
Movement	7	8	9		10	11		12
	L	T	R		L	Т		R
Volume (veh/h)	9		6					
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
Hourly Flow Rate, HFR (veh/h)	9	0	6		0	0		0
Percent Heavy Vehicles	0	0	0		0	0		0
Percent Grade (%)		0				0		
Flared Approach		Y				N		
Storage		1				0		
RT Channelized			0			1		0
Lanes	0	0	0		0	0		0
Configuration		LR						
Delay, Queue Length, ar	nd Level of Ser	vice						0
Approach	Eastbound	Westbound		Northboun	d	5	Southbound	ì
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (veh/h)		10		15				
C (m) (veh/h)		1289		862				
v/c		0.01		0.02	+			
95% queue length		0.02	-	0.05		<del> </del>		
				11.1	+			<del>                                     </del>
Control Delay (s/veh)		7.8			-	<del> </del>		
LOS		Α		В				1
Approach Delay (s/veh)				11.1				
Approach LOS				В				

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	Τ\	WO-WAY STOF	CONTR	OL SU	JMM	IARY				
General Information			Site Ir	nform	atio	n				
Analyst	Alex Flem	ning	Interse	ction			Hwy. 26 8	Arrow	head	Rd.
Agency/Co.		rier & Associates	Jurisdi	ction			MTO			
Date Performed	6/18/2007	7	Analys	is Year			2007 Exis	ting Vo	olume	S
Analysis Time Period	P.M. Pea	k Hour								
Project Description 119	-2528						39-5925			
East/West Street: Highw	ay 26					Arrowhea	nd Road			
Intersection Orientation:	East-West		Study F	Period (	(hrs):	0.25				
Vehicle Volumes an	d Adjustmen									
Major Street		Eastbound					Westbou	nd		2
Movement	1	2	3			4	5 T	_	-	6 R
	L	T	R			11	513	-+		<u> </u>
Volume (veh/h)	1.00	398 1.00	1.00	_	_	1.00	1.00	-		1.00
Peak-Hour Factor, PHF Hourly Flow Rate, HFR	1.00		1					$\dashv$		
(veh/h)	0	398	12			11	513			0
Percent Heavy Vehicles	0					9				-
Median Type				Undi	vided					
RT Channelized			0							0
Lanes	0	1	1			0	1			0
Configuration		T	R			LT				
Upstream Signal		0					0			
Minor Street		Northbound					Southbou	ınd		
Movement	7	8	9			10	11			12
	L	T	R			L	Т			R
Volume (veh/h)	7		5					_		
Peak-Hour Factor, PHF	1.00	1.00	1.00			1.00	1.00	_		1.00
Hourly Flow Rate, HFR (veh/h)	7	0	5			0	0			0
Percent Heavy Vehicles	0	0	0			0	0			0
Percent Grade (%)		0					0			
Flared Approach		Y					N			
Storage		1					0			
RT Channelized			0							0
Lanes	0	0	0			0	0			0
Configuration		LR								
Delay, Queue Length, ar	nd Level of Serv	rice								
Approach	Eastbound	Westbound		Northb	ound		5	Southbo		
Movement	1	4	7	8		9	10	11	1	12
Lane Configuration		LT		LR						
v (veh/h)		11		12						
C (m) (veh/h)		1112		506	ĵ					
v/c		0.01		0.0	2					
95% queue length		0.03		0.0	7					
Control Delay (s/veh)		8.3		14.	6					
LOS	-	Α		В						
Approach Delay (s/veh)				14.0						
Approach LOS		. <del></del>		В						
TPPIOACII LOG		21								

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		TV	VO-WAY STOR	CONTR	OL SU	MN	IARY				
General Information	1			Site I	nform	atio	n				
Analyst		Alex Flem	ning	Interse	ection		Mar.	Hwy. 26	& Hidde	n Lal	ke Rd.
Agency/Co.			ier & Associates	Jurisd	iction			МТО			
Date Performed		6/26/2007	•	Analys	sis Year			2014 Futu	ıre Bac	kgrou	ınd
Analysis Time Period		A.M. Pear	k Hour								
Project Description 119	9-2528										
East/West Street: Highw	ay 26			North/S	South St	reet	: Hidden L	ake Road			
Intersection Orientation:		West		Study	Period (I	hrs):	0.25				
Vehicle Volumes an	d Adj	ustmen	ts		-						
Major Street			Eastbound					Westbou	ınd		
Movement		1	2	3			4	5			6
		L	T	R			L	T			R
Volume (veh/h)			391	0			2	339			
Peak-Hour Factor, PHF		1.00	1.00	1.00	)		1.00	1.00		1	1.00
Hourly Flow Rate, HFR (veh/h)		0	391	0			2	339			0
Percent Heavy Vehicles		0	3				0				****
Median Type					Undiv	rided	1				
RT Channelized				0							Q
Lanes		0	1	1			0	1			0
Configuration			T	R			LT				
Upstream Signal			0					0			
Minor Street			Northbound					Southbou	ınd		
Movement		7	8	9			10	11			12
		L	Т	R			L	Т			R
Volume (veh/h)		2		3							
Peak-Hour Factor, PHF		1.00	1.00	1.00	)		1.00	1.00		1	.00
Hourly Flow Rate, HFR (veh/h)		2	0	3			0	0			0
Percent Heavy Vehicles		0	0	0			0	0			0
Percent Grade (%)			0					0			
Flared Approach	- 1		N					N			
Storage			1					0			
RT Channelized				0							0
Lanes		0	0	0			0	0			0
Configuration			LR								
Delay, Queue Length, ar	ıd Lev	el of Serv	ice								
Approach	Eas	tbound	Westbound		Northbo	und			Southbo	ound	
Movement		1	4	7	8		9	10	11		12
Lane Configuration			LT		LR						
v (veh/h)			2		5					$\neg$	
C (m) (veh/h)			1179		517						
v/c			0.00		0.01					$\neg$	
95% queue length			0.01		0.03					$\dashv$	
Control Delay (s/veh)			8.1		12.0	_				$\dashv$	
LOS			A. A.		12.0 B				<u> </u>	$\overline{}$	
Approach Delay (s/veh)					12.0						
Approach LOS					В			1			

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	T\	WO-WAY STOP	CONTR	OL SU	MARY			
General Information	1		Site I	nforma	tion			
Analyst	Alex Flen	ning	Interse	ection		Hwy. 26 8	& Hidden L	ake Rd.
Agency/Co.		zier & Associates	Jurisdi	ction		MTO		
Date Performed	6/26/200	7	Analys	is Year		2014 Futu	ıre Backgı	round
Analysis Time Period	P.M. Pea	k Hour						
Project Description 119	9-2528							70
East/West Street: Highw					eet: <i>Hidden</i>	Lake Road		
Intersection Orientation:	East-West		Study I	Period (h	rs): 0.25			
Vehicle Volumes an	d Adjustmen							
Major Street		Eastbound	T 0		4	Westbou	ind	
Movement	1	2 	3 R		4	5 T		6 R
Volume (veh/h)	<del></del>	566	3		L	713		
Peak-Hour Factor, PHF	1.00	1.00	1.00	<del>.  </del>	1.00	1.00		1.00
Hourly Flow Rate, HFR			_					
(veh/h)	0	566	3		3	713		0
Percent Heavy Vehicles	0				0			
Median Type				Undivid	ded			
RT Channelized			0					0
Lanes	0	1	1		0	1		0
Configuration		T	R		LT			
Upstream Signal		0				0		
Minor Street		Northbound				Southbou	und	
Movement	7	8	9		10	11		12
	L.	Т	R		L	Ţ		R
Volume (veh/h)	1		0					
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
Hourly Flow Rate, HFR (veh/h)	1	0	0		0	0		0
Percent Heavy Vehicles	0	0	0		0	0		0
Percent Grade (%)		0				0		
Flared Approach		N				N		
Storage		1				0		
RT Channelized			0					0
Lanes	0	0	0		0	0		0
Configuration		LR						
Delay, Queue Length, ar	nd Level of Serv	/ice						
Approach	Eastbound	Westbound		Northbou	ınd	5	Southboun	d
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (veh/h)		3		1				
C (m) (veh/h)		1013		182				
v/c		0.00		0.01				
95% queue length		0.01		0.02			0	
Control Delay (s/veh)		8.6		24.9				
LOS		Α		С				
Approach Delay (s/veh)	-			24.9				
Approach LOS		<u>=</u>		С		1		
						_1		

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General Information			Site Ir	formati	on			
	Alex Fler	ning	Interse		<u> </u>	Hwy. 26 8	Arrowhea	ad Rd
Analyst Agency/Co.		zier & Associates	Jurisdie			MTO	71110111101	10.
Date Performed	6/26/200		Analys			2014 Futu	re Backar	ound
Analysis Time Period	A.M. Pea		- I midny d					Acceptants
Project Description 119							- 1:	
ast/West Street: Highw			North/S	outh Stree	et: Arrowhe	ad Road		
ntersection Orientation:				eriod (hrs		44,1044		
		4-	jotata) i	Ollow Villa			10-10-	
/ehicle Volumes and	a Aajustmer					Westbou	nd	
Major Street		Eastbound 2	3		4	T 5	TIQ T	6
Movement	11	<del>                                     </del>	R		<del></del>	T		R
/olume (veh/h)		361	26		19	325		
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
lourly Flow Rate, HFR			-i			1		
veh/h)	0	361	26		19	325		0
Percent Heavy Vehicles	0		T		0	. ++		200
ledian Type				Undivide	ed			
RT Channelized			0					0
anes	0	1	1	04.1	0	1		0
Configuration		T	R		LT			
Jpstream Signal		0				0		
linor Street		Northbound				Southbou	nd	
lovement	7	8	9		10	11		12
io voltione	T L	Ť	R		L	, т		R
olume (veh/h)	13		17			1		
eak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
lourly Flow Rate, HFR	13	0	17		0	0		0
veh/h)								
ercent Heavy Vehicles	0	0	0		0	0		0
ercent Grade (%)		0				0		
lared Approach		Y				N		
Storage		1				0		
RT Channelized			0					0
anes	0	0	0		0	0		0
Configuration		LR	1					
elay, Queue Length, an	d Level of Ser					***		
pproach	Eastbound	Westbound		Northboun	ıd		Southboun	d
	1	4	7	8	9	10	11	12
lovement					+ -	<del>  ''</del>		<del>  ''</del>
ane Configuration		LT		LR		<del> </del>		+-
(veh/h)		19		30				
(m) (veh/h)		1183		900		ļ		
/c		0.02		0.03				
5% queue length		0.05		0.10				
Control Delay (s/veh)		8.1		12.2				
OS		A		В	<del>                                     </del>	1		1
	2002	+		12.2		<del>-</del>		
pproach Delay (s/veh)						<del> </del>		
pproach LOS				В		1		

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		VO-WAY STOP						
General Information				format	ion	T		167
Analyst	Alex Flem		Interse			Hwy. 26 & A	Arrownea	a Rą.
Agency/Co.		ier & Associates	Jurisdic			MTO	- Doolean	sund.
Date Performed	6/26/2007		Analysi	s year		2014 Futur	е Баскуго	ouria
Analysis Time Period	P.M. Peal	k Hour						
Project Description 119-			h		- t. A	ad David		
East/West Street: Highwa					et: Arrowhe	aa Roaa		
ntersection Orientation:	East-West		Study P	eriod (hr	s): <i>0.2</i> 5			
/ehicle Volumes and	l Adjustmen							
Major Street		Eastbound				Westboun	d	
Movement	1	2	3		4	5		6
	L	Ţ	R		L	T 707		R
/olume (veh/h)		546	16		20	707	_	1.00
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		
Hourly Flow Rate, HFR veh/h)	0	546	16		20	707		0
Percent Heavy Vehicles	0				9			
Median Type				Undivid	ed			
RT Channelized			0					0
anes	0	1	1		0	1		0
Configuration		T	R		LT			
Jpstream Signal		0				0		
/linor Street		Northbound				Southbour	nd	
Movement	7	8	9		10	11		12
	L	Т	R		L	Т		R
/olume (veh/h)	11		16					
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00
lourly Flow Rate, HFR veh/h)	11	0	16		0	0		0
Percent Heavy Vehicles	0	0	0		0	0		0
Percent Grade (%)		0				0		
Flared Approach	1	Y				N		
Storage		1				0		
RT Channelized	-		0			1		0
	0	0	0		0	0		0
_anes Configuration	<del>                                     </del>	LR	† – Ť	-+				
	d l aval af Or-					+		
Delay, Queue Length, an		Westbound		Northbou	nd	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	outhboun	d
Approach	Eastbound			8	9	10	11	12
Movement	1	4	7		— <u> </u>	1 10		12
ane Configuration		LT		LR		++		
v (veh/h)		20		27				<b>_</b>
C (m) (veh/h)		975		434				
r/c		0.02		0.06				
95% queue length		0.06		0.20				
Control Delay (s/veh)		8.8	i	17.9				1
		A	<del></del>	C	_	1		
LOS		<b></b>	<u> </u>	17.9		+		
Approach Delay (s/veh)		-				-		
Approach LOS				С				

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		VO-WAY STOP							
General Information				formation	on				
Analyst	Alex Flem		Interse				l Rd. & Alta		
Agency/Co.		er & Associates	Jurisdi			Town of The Blue Mountains			
Date Performed	6/26/2007		Analys	is Year		2014 Future Background			
Analysis Time Period	A.M. Peak	( Hour							
	)-2528								
East/West Street: Alta R					t: Arrowhe	ad Road			
ntersection Orientation:	North-South		Study F	eriod (hrs)	: 0.25				
Vehicle Volumes an	d Adjustment								
Major Street		Northbound				Southbou	<u>nd</u>		
Movement	1	2	3		4	5		6	
	L	T	R		<u> </u>	T		R	
/olume (veh/h)	7	34	1.00		4.00	1.00		16 1.00	
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	7.00		.00	
Hourly Flow Rate, HFR veh/h)	7	0	9		0	0		0	
Percent Heavy Vehicles	29				0				
Median Type				Undivide	d				
RT Channelized			0					0	
_anes	0	1	0		0	1		0	
Configuration	LT							TR	
Jpstream Signal		0				0			
Minor Street		Eastbound				Westbou	nd		
Movement	7	8	9		10	11		12	
	L	Т	R		L	T		R	
Volume (veh/h)	7		9						
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00	1	1.00	
Hourly Flow Rate, HFR	0	29	16		7	34		0	
(veh/h)								0	
Percent Heavy Vehicles	0	0	11		0	0		U	
Percent Grade (%)		0				0			
Flared Approach		N				N			
Storage		0				0			
RT Channelized			0					0	
Lanes	0	0	0		0	0		0	
Configuration		LR							
Delay, Queue Length, ar	nd Level of Serv	ice							
Approach	Northbound	Southbound		Westbound	d		Eastbound		
Movement	1	4	7	8	9	10	11	12	
_ane Configuration	LT				1		LR		
	7				<del>                                     </del>	1	16		
/ (veh/h)					<del>                                     </del>	<del> </del>	967		
C (m) (veh/h)	1406				-	<b> </b>			
r/c	0.00						0.02		
95% queue length	0.02						0.05		
Control Delay (s/veh)	7.6						8.8		
OS	Α						Α		
Approach Delay (s/veh)					*		8.8		
Approach LOS						1	Α		
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		ΤV	VO-WAY STOP	CONTR	OL SI	JMN	IARY				
General Information	1		•	Site I	nform	atio	n				
Analyst	Al	ex Flem	ing	Interse	ection			Arrowhe	ad Ro	d. & Alta	Rd.
Agency/Co.	1707	27.274	er & Associates	Jurisdi				Town of The Blue Mountains			
Date Performed		26/2007		Analys	is Year			2014 Fu	ture E	Backgro	und
Analysis Time Period		M. Peak	Hour								
Project Description 119											
East/West Street: Alta F				North/South Street: Arrowhead Road Study Period (hrs): 0.25							
Intersection Orientation:				Study	eriod (	hrs):	0.25				w
Vehicle Volumes an	d Adjus	stment						0 11			
Major Street		4	Northbound	1 2				Southbound		т—	6
Movement		1		3 R			4 L	5 T		+	6 R
Volume (veh/h)		3	24	<u> </u>		-		27		+	3
Peak-Hour Factor, PHF	<del></del>	1.00	1.00	1.00		-	1.00	1.00		+	1.00
Hourly Flow Rate, HFR			-i							+-	
(veh/h)		12	0	7			0	0			0
Percent Heavy Vehicles		0					0	:			:##:
Median Type					Undi	vided					
RT Channelized				0							0
Lanes		0	1	0			0	1			Q
Configuration		LT									TR
Upstream Signal			0					0			
Minor Street			Eastbound					Westbo	und		
Movement		7	8	9			10	11			12
		L	Т	R			L	Т			R
Volume (veh/h)		12	100	7			4.00	4.00			( 00
Peak-Hour Factor, PHF		1.00	1.00	1.00			1.00	1.00	1.00		1.00
Hourly Flow Rate, HFR (veh/h)		0	27	3			3	24			0
Percent Heavy Vehicles		0	0	0			0	0			0
Percent Grade (%)			0					0			
Flared Approach			N					N			
Storage			0					0			
RT Channelized				0							0
Lanes		0	0	0			0	0			0
Configuration			LR								
Delay, Queue Length, ar											
Approach	Northbe	ound	Southbound		Westbo	_			East	tbound	
Movement	1		4	7	8		9	10		11	12
Lane Configuration	LT									LR	
v (veh/h)	3									19	
C (m) (veh/h)	159	6							T :	987	
v/c	0.00	0							1	0.02	
95% queue length	0.01	1								0.06	
Control Delay (s/veh)	7.3								_	8.7	
LOS	7.0 A								+-	A	
Approach Delay (s/veh)					!					3.7	
									_	A.	
Approach LOS										^	

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	TV	VO-WAY STOP	CONTR	OL SU	MM	ARY				
General Information			Site Ir	nform	atior	1				
Analyst	Alex Flem	ning	Interse	ction			Hwy. 26 8	& Arrowhe	ad Rd.	
Agency/Co.		ier & Associates	Jurisdi				MTO			
Date Performed	6/27/2007		Analys	is Year			2014 Tota	s		
Analysis Time Period	A.M. Peal	k Hour				12				
Project Description 119	-2528									
East/West Street: Highwo	ay 26		North/South Street: Arrowhead Road Study Period (hrs): 0.25							
Intersection Orientation:			Study	rerioa (	nrs):	0.25				
Vehicle Volumes and	d Adjustmen						Westbou	-d		
Major Street  Movement	1	Eastbound 2	T 3			4	T 5	IIQ	6	
viovement	<del>                                     </del>	T	R			<del>-</del>	<del>                                     </del>		R	
Volume (veh/h)	<del> </del>	364	26			21	328			
Peak-Hour Factor, PHF	1.00	1.00	1.00			1.00	1.00		1.00	
Hourly Flow Rate, HFR	0	364	26			21	328		0	
veh/h)		304	20							
Percent Heavy Vehicles	0				., .	0				
Median Type				Undiv	/ided		·			
RT Channelized			0				<b>_</b>		0	
anes	0	1	1			0	1		Q	
Configuration		T	R			LT	-	_		
Jpstream Signal		0					0			
Minor Street	7	Northbound	T 0			40	Southbound		12	
Movement	7	8 T	9 R	-		10 L	11 T		R	
(alives a (ivala /la)	13		19							
/olume (veh/h) Peak-Hour Factor, PHF	1.00	1.00	1.00		-	1.00	1.00		1.00	
Hourly Flow Rate, HFR										
veh/h)	13	0	19			0	0		0	
Percent Heavy Vehicles	0	0	0			0	0	0		
Percent Grade (%)		0					0			
Flared Approach		Y					N			
Storage		1					0			
RT Channelized			0						0	
anes	0	0	0			0	0		0	
Configuration		LR								
Delay, Queue Length, an	d Level of Serv	ice								
Approach	Eastbound	Westbound		Northbo	ound		3	Southbour	nd	
/lovement	1	4	7	8		9	10	11	12	
ane Configuration		LT		LR						
(veh/h)		21		32	$\dashv$					
C (m) (veh/h)		1180		943	_		1			
/c		0.02		0.03	$\rightarrow$					
95% queue length		0.05		0.11			<b> </b>			
		8.1		12.2			<b>-</b>		_	
Control Delay (s/veh)					-					
os		Α		B	igspace		ļ			
Approach Delay (s/veh)	77.			12.2	<u>'</u>		ļ			
Approach LOS				В			<u></u>			

General Information			Site Ir	nforma	tion				
Analyst	Alex Flem	ina	Interse			Hwy. 26 8	Arrowhe	ad Rd.	
Agency/Co.		er & Associates	Jurisdie	100 N		MTO			
Date Performed	6/26/2007		Analys			2014 Total Volume			
Analysis Time Period	P.M. Peak	Hour							
	-2528								
East/West Street: Highw			North/S	outh Str	eet: Arrowhe	ad Road			
ntersection Orientation:			Study F	Period (h	rs): 0.25				
/ehicle Volumes and	d Adjustment	s							
Major Street	1	Eastbound				Westbou	nd		
Movement	1	2	3		4	5		6	
	L	Т	R		L	T		R	
/olume (veh/h)		560	16		22	710			
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00	
lourly Flow Rate, HFR veh/h)	0	560	16		22	710		0	
Percent Heavy Vehicles	0				9			22	
Median Type				Undivi	ded				
RT Channelized			0					0	
anes	0	1	1		0	1		0	
Configuration		T	R		LT				
Jpstream Signal		0				0			
/linor Street		Northbound				Southbou	Southbound		
Movement	7	8	9		10	11		12	
	L	Т	R		L	T		R	
/olume (veh/h)	11		18						
eak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00	
lourly Flow Rate, HFR veh/h)	11	0	18		0	0		0	
Percent Heavy Vehicles	0	0	Q		0	0		0	
Percent Grade (%)		0				0			
Flared Approach		Y				N			
Storage		1				0			
RT Channelized			0					0	
anes	0	0	0		0	0		0	
Configuration		LR							
Delay, Queue Length, an	d Level of Serv			•					
pproach	Eastbound	Westbound		Northbou	und	1 8	Southbour	nd	
Novement	1	4	7	8	9	10	11	12	
ane Configuration	'	LT		LR	<del>                                     </del>	1		_	
		22		29		<u> </u>		_	
(veh/h)				453		-			
(m) (veh/h)		964							
/c		0.02		0.06				_	
5% queue length		0.07		0.20				-	
Control Delay (s/veh)		8.8		17.8					
.OS		Α		С					
pproach Delay (s/veh)		-		17.8					

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		TV	VO-WAY STOP	CONTR	OL SU	JMN	IARY				
General Information				Site I	nform	atio	n				
Analyst		Alex Flem	ina	Interse	Intersection				& Hidd	en Lai	ke Rd.
Agency/Co.		C.F. Crozi	er & Associates	Jurisdi				MTQ			
Date Performed		6/27/2007		Analys	is Year			2014 Tot	al Volu	mes	
Analysis Time Period		A.M. Peak	Hour								
Project Description 119	-2528				-						
East/West Street: Highw				North/S	South S	treet	: Hidden L	.ake Road			
Intersection Orientation:		West		Study I	Period (	hrs):	0.25				
Vehicle Volumes an	d Adj	ustment	S								
Major Street	$\top$					Westbound					
Movement		1	2	3			4	5			6
		L	T	R			L	T			R
Volume (veh/h)			393	2			5	342			
Peak-Hour Factor, PHF		1.00	1.00	1.00			1.00	1.00			1.00
Hourly Flow Rate, HFR (veh/h)		0	393	2			5	342			0
Percent Heavy Vehicles		0					0				
Median Type					Undiv	⁄idea					
RT Channelized				0							0
Lanes		0	1	1			0	1			Q
Configuration			T	R			LT				
Upstream Signal			0					0			
Minor Street			Northbound					Southbound			
Movement		7	8	9		10		11			12
		L	Т	R			L	T			R
Volume (veh/h)		4		6							
Peak-Hour Factor, PHF		1.00	1.00	1.00			1.00	1.00			.00
Hourly Flow Rate, HFR (veh/h)		4	0	6			0	0			0
Percent Heavy Vehicles	i	0	0	0			0	0			0
Percent Grade (%)			0	****				0			
Flared Approach			N				*	N			
Storage	1		1					0			
RT Channelized				0							0
Lanes		0	0	0			0	0			Q
Configuration			LR								
Delay, Queue Length, ar	nd Leve	el of Servi	ce								
Approach	East	bound	Westbound		Northbo	ound			Southb	ound	
Movement		1	4	7	8		9	10	1	1	12
Lane Configuration			LT		LR						
v (veh/h)			5		10						
C (m) (veh/h)			1175		511				Ī		
v/c			0.00		0.02				1		
95% queue length			0.01		0.06	_			1		
Control Delay (s/veh)			8.1		12.2		-		+		
			A A		12.2 B				<del> </del>		
LOS								-	1		
Approach Delay (s/veh)					12.2						
Approach LOS		: <del>****</del>		В			L				

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	TV	WO-WAY STOP	CONTR	OL SUM	//ARY					
General Information			Site In	nformatio	on .					
Analyst	Alex Flen	nina	Interse	ction		Hwy. 26 & Hidden Lake Rd.				
Agency/Co.		rier & Associates	Jurisdi	ction		MTO				
Date Performed	6/26/2007	7	Analys	is Year		2014 Total Volumes				
Analysis Time Period	P.M. Pea	k Hour								
Project Description 119	-2528									
East/West Street: Highw			North/South Street: Hidden Lake Road							
Intersection Orientation:	East-West		Study F	Period (hrs)	0.25					
Vehicle Volumes an	d Adjustmen	ts								
Major Street		Eastbound				Westbou				
Movement	11	2	3		4	5		6		
	L	T	R			T 740		R		
Volume (veh/h)	100	568	5		6	716		1.00		
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00				
Hourly Flow Rate, HFR (veh/h)	0	568	5		6	716		0		
Percent Heavy Vehicles	0	***			0	-				
Median Type				Undivide	d					
RT Channelized			0					0		
Lanes	0	1	1		Q	1		0		
Configuration		T	R		LT					
Upstream Signal		0				0				
Minor Street		Northbound					ınd			
Movement	7	8	9		10	11		12		
	L	Т	R		L	T		R		
Volume (veh/h)	3		4							
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00		1.00		
Hourly Flow Rate, HFR (veh/h)	3	0	4		0	0		0		
Percent Heavy Vehicles	0	0	Q		0	0		0		
Percent Grade (%)		0	-			0				
Flared Approach		N				N				
Storage		1				0				
RT Channelized			0					0		
Lanes	0	0	0		0	0		0		
Configuration		LR								
Delay, Queue Length, ar	nd Level of Serv	/ice								
Approach	Eastbound	Westbound		Northboun	d	5	outhbound			
Movement	1	4	7	8	9	10	11	12		
Lane Configuration		LT		LR						
v (veh/h)		6		7						
C (m) (veh/h)		1010		288						
v/c		0.01		0.02						
95% queue length		0.02		0.07						
		8.6		17.8		1				
Control Delay (s/veh)				C C		<del>                                     </del>		+		
LOS		Α		1		+	<u> </u>			
Approach Delay (s/veh)				17.8		ļ				
Approach LOS			С							

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	TV	O-WAY STOP	CONTRO	L SU	MMARY						
General Information			Site In	form	ation						
Analyst	Alex Flem	ing	Interse	ction		Arrowhead	d Rd. & Alta	a Rd.			
Agency/Co.		er & Associates	Jurisdio	Jurisdiction			Town of The Blue Mountains				
Date Performed	6/27/2007		Analysi	s Year		2014 Tota	2014 Total Volumes				
Analysis Time Period	A.M. Peak	Hour									
Project Description 119	-2528							8 8			
ast/West Street: Alta Re	oad		North/South Street: Arrowhead Road								
ntersection Orientation:	North-South		Study P	eriod (	hrs): 0.25						
/ehicle Volumes and	d Adjustment										
Major Street		Northbound				Southbou	nd	-			
Movement	1	2	3		4	5 T		6 R			
- W	L L	T	R		L	29		18			
/olume (veh/h)	10	34	1.00	_	1.00	1.00	- 2	1.00			
Peak-Hour Factor, PHF	1.00	1.00									
lourly Flow Rate, HFR veh/h)	9	0	11		0	0		0			
Percent Heavy Vehicles	29				0						
Median Type				Vay Le	ft Turn Lane						
RT Channelized			0					Q			
anes	0	1	0		0	1		0 TR			
Configuration	LT										
Jpstream Signal		0					0				
/linor Street		Eastbound					Westbound				
/lovement	7	8	9		10	11		12			
	L	Т	R		L	T		R			
/olume (veh/h)	9		11		1.00	1.00		1.00			
Peak-Hour Factor, PHF	1.00	1.00	1.00		1.00	1.00					
lourly Flow Rate, HFR veh/h)	0	29	18		10	34		0			
Percent Heavy Vehicles	0	0	11		0	0		0			
Percent Grade (%)		0				0					
Flared Approach		N				N					
Storage		0				0					
RT Channelized			0					0			
anes	0	0	0		0	0		0			
Configuration		LR									
Delay, Queue Length, ar	nd Level of Serv	ice									
Approach	Northbound	Southbound		Westb	ound		Eastbound				
/lovement	1	4	7	8	9	10	11	12			
ane Configuration	LT						LR				
(veh/h)	10						20				
C (m) (veh/h)	1404						935				
/c	0.01						0.02				
95% queue length	0.02						0.07				
Control Delay (s/veh)	7.6						8.9				
OS	A A						Α				
							8.9				
Approach Delay (s/veh)						_	A				
Approach LOS			L								

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	TV	NO-WAY STOR	CONTR	OL SL	JMN	IARY				
General Information	l	·	Site I	nform	atio	n				
Analyst	Alex Flen	ning	Interse	ection	r		Arrowhea	d Rd. & Alt	a Rd.	
Agency/Co.	C.F. Croz	ier & Associates	Jurisdi	ction			Town of The Blue Mountain			
Date Performed	6/27/2007		Analys	is Year			2014 Tota	al Volumes		
Analysis Time Period	P.M. Pea	k Hour							-	
Project Description 119										
East/West Street: Alta R						: Arrowhe	ad Road			
Intersection Orientation:			Study F	Period (	hrs):	0.25				
Vehicle Volumes an	<u>d Adjustmen</u>						Southbou	ınd	1172	
Major Street Movement	1	Northbound 2	Northbound 2 3		4		5	ing	6	
viovement	+ +	T	R		-	<del>-</del>	Ť		R	
Volume (veh/h)	5	24	<del>  ``</del>		_		27	-	5	
Peak-Hour Factor, PHF	1.00	1.00	1.00			1.00	1.00		1.00	
Hourly Flow Rate, HFR	14	0	10			0	0		0	
(veh/h)		_								
Percent Heavy Vehicles	0	-		Undiv	idod	0	(20) (20)			
Median Type	_	-т	1 0	Ullak	naea		_		0	
RT Channelized		1	0	-	_	0	1	_	0	
anes	O LT		- ·			U	<del> </del>		TR	
Configuration Upstream Signal		0	_				0			
	+						Westbound			
Minor Street Movement	7	Eastbound 8	<b>I</b> 9		-	10	11	Tid T	12	
viovement	<del>                                     </del>	T	R	-		L	<del>                                     </del>	_	R	
/olume (veh/h)	14	<del></del>	10				<del></del>			
Peak-Hour Factor, PHF	1.00	1.00	1.00			1.00	1.00	1.00 1.0		
Hourly Flow Rate, HFR	0	27	5			5	24		0	
veh/h)	0	0	0			0	0	-	0	
Percent Heavy Vehicles Percent Grade (%)	- 0	0	0			U	0			
	_	T N		-			T N			
Flared Approach	-	0					0			
Storage RT Channelized			0		_			_	0	
	0	0	0			0	0		0	
_anes Configuration	<del>-                                    </del>	LR	+			-	<del>                                      </del>			
	d Lavel of Com									
Delay, Queue Length, an	Northbound	Southbound		Westbo	und		T	Eastbound		
Approach Movement	1	4	7	8	Junu	9	10	11	12	
ane Configuration	LT	**	- '	<del>_</del>				LR	1 '-	
(veh/h)	5						<del> </del>	24	1	
	1593						<b>-</b>	985	<del>                                     </del>	
C (m) (veh/h)	0.00			-				0.02	<b>+</b>	
					_			0.02	1	
95% queue length	0.01		-					8.7	1	
Control Delay (s/veh)	7.3								<del> </del>	
.os	Α							A		
pproach Delay (s/veh)	122							8.7		
Approach LOS								Α		

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