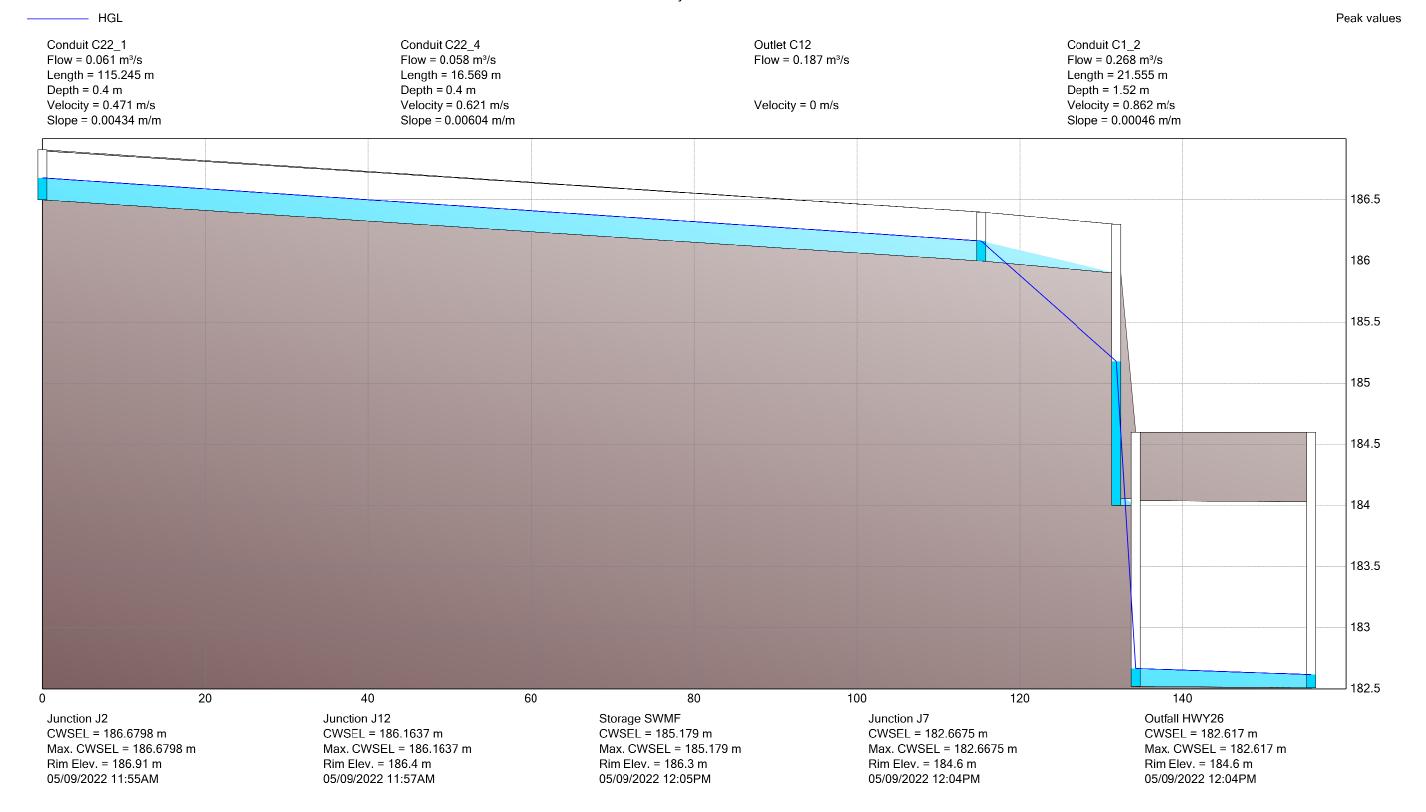
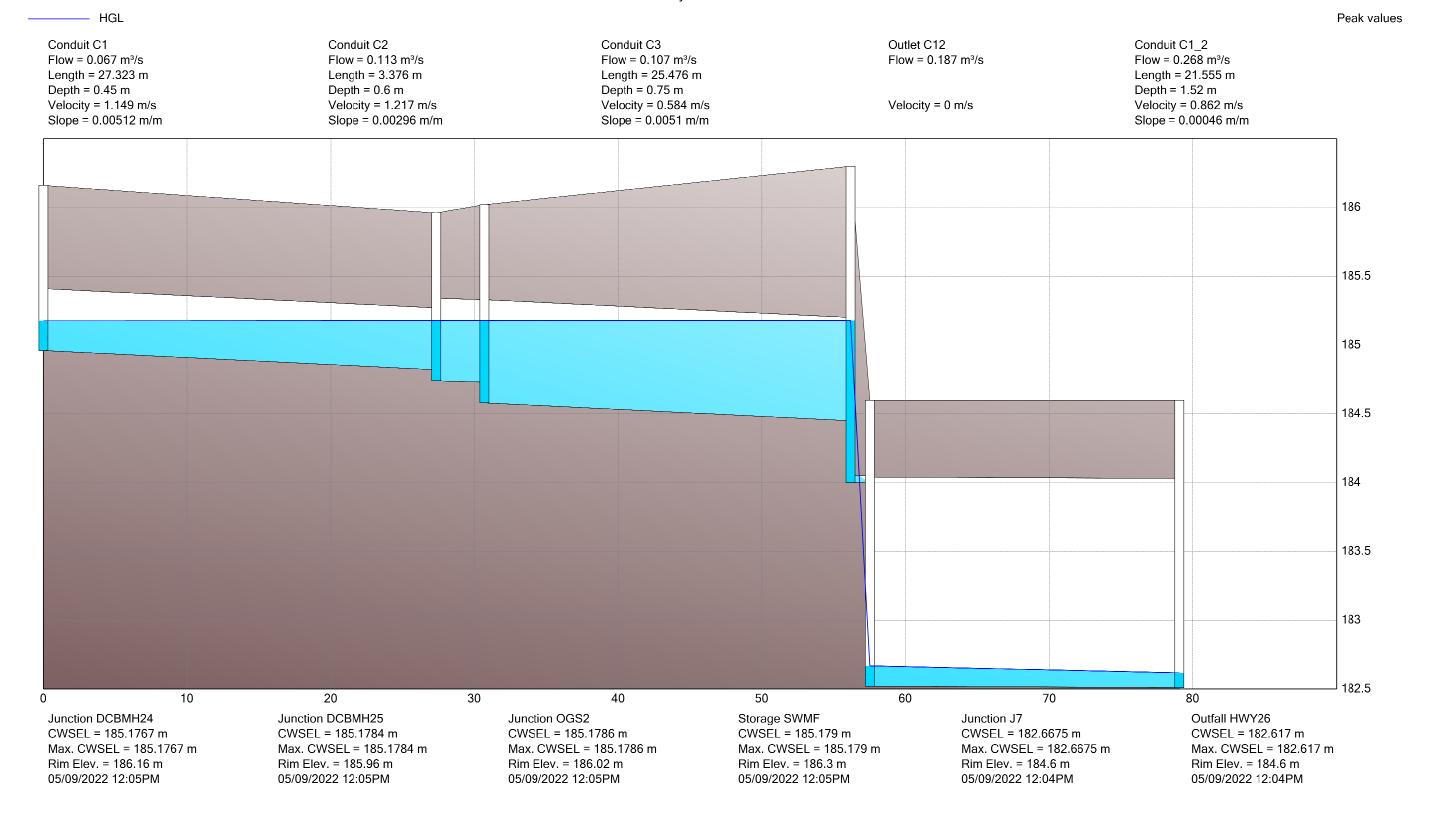
# Profile 4 Swale to Hwy 26 Outlet



## Profile 5 DCBMH24 to Hwy 26 Outlet



# Profile 6 DCB01 to Hwy 26 Outlet

						-						
	HGL											Peak values
Links:	C1_2 Q=0.268 m³/s L=21.555 m D=1.52 m V=0.862 m/s S=0.00046 m/m	C12 Q=0.187 m³/s V=0 m/s	C13 Q=0.296 m³/s L=10.911 m D=0.75 m V=1.02 m/s S=0.00275 m/m	C29 Q=0.116 m <sup>3</sup> /s L=14.809 m D=0.6 m V=1.017 m/s S=0.00338 m/m	C28 Q=0.121 m³/s L=20.311 m D=0.6 m V=0.966 m/s S=0.00295 m/r	C27 Q=0.114 m <sup>2</sup> L=34.163 m D=0.6 m V=0.962 m/ m S=0.00293	n L=31.39 D=0.45 /s V=1.20	01 m³/s Q 96 m L 5 m D 1 m/s V	C25 Q=0.088 m³/s =26.358 m O=0.375 m /=1.193 m/s G=0.00304 m/m	C24 Q=0.057 m³/s L=17.357 m D=0.375 m V=0.737 m/s S=0.00288 m/m	C23 Q=0.048 m³/s L=34.083 m D=0.375 m V=0.742 m/s S=0.00293 m/m	C22 Q=0.042 m <sup>3</sup> /s L=12.071 m D=0.375 m V=0.783 m/s S=0.00331 m/m
												186.5
												185.5
												185
												184.5
												184
												183.5
												183
	20	40	00	00	100	100	140	100	0 40	0 200	220	182.5
0 Nodes:	20 H=182.617 m M=182.617 m R=184.6 m	J7 H=182.6675 m M=182.6675 m R=184.6 m	60 SWMF H=185.179 m M=185.179 m R=186.3 m	H=185.1785 m M=185.1785 m	100 MH08 H=185.1788 m M=185.1788 m R=186.88 m	120 DCBMH07 H=185.1792 m M=185.1792 m R=186.74 m	140 DCBMH06 H=185.1791 m M=185.1791 m R=186.72 m	DCBMH05 H=185.3388 M=185.3388 R=186.73 m	DCBMH04 m H=185.499 m M=185.499	DCBMH03 2 m H=185.5235 2 m M=185.5235	DCBMH02 m H=185.5807 m m M=185.5807 m	DCB01 H=185.6072 m M=185.6072 m R=186.84 m

#### Post Development - 100yr 24hr SCS Type II - PCSWMM Output

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EPA STORM WATER MANAGEMENT MODEL - VERSION 5.2 (Build 5.2.4)
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\*\*\*\*\*\*\*\*\*\*
Element Count
\*\*\*\*\*\*\*\*

Number of rain gages ..... 14
Number of subcatchments ... 27
Number of nodes ..... 34
Number of links ..... 32
Number of pollutants .... 0
Number of land uses .... 0

Name	Data Source		ecording nterval
25mm	25mm	INTENSITY	10 min.
Chicago 4h 100Yr	Chicago 4h 100Yr	INTENSITY	5 min.
Chicago_4h_10Yr	Chicago_4h_10Yr	INTENSITY	5 min.
Chicago_4h_25Yr	Chicago_4h_25Yr	INTENSITY	5 min.
Chicago_4h_2Yr	Chicago_4h_2Yr	INTENSITY	5 min.
Chicago_4h_50Yr	Chicago_4h_50Yr	INTENSITY	5 min.
Chicago_4h_5yr	Chicago_4h_5yr	INTENSITY	5 min.
SCS_Type_II_108.79mm	_25Yr SCS_Type_II_108.79mm_25Yr	INTENSI	TY 6 min.
SCS_Type_II_121.11mm	_50Yr SCS_Type_II_121.11mm_50Yr	INTENSI	TY 6 min.
SCS_Type_II_133.1mm_	100yr SCS_Type_II_133.1mm_100yr	INTENSI	TY 6 min.
SCS_Type_II_59.84mm_	2Yr SCS_Type_II_59.84mm_2Yr	INTENSITY	6 min.
SCS_Type_II_79.64mm_	5Yr SCS_Type_II_79.64mm_5Yr	INTENSITY	6 min.
SCS_Type_II_92.51mm_	10Yr SCS_Type_II_92.51mm_10Yr	INTENSIT	Y 6 min.
Timmins_Storm_ $(0-25)$	Timmins_Storm_(0-25)	INTENSITY	60 min.

Name	Area		%Imperv	%Slope Rain Gage Outlet
A1	0.53			2.0000 SCS Type II 133.1mm 100yr J11
A10	0.03	16.08	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH22
A11	0.05	35.13	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCB13
A12	0.04	28.62	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH14
A13	0.03	23.40	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH15
A14	0.05	28.33	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH16
A15	0.05	26.70	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCB09
A16	0.04	27.53	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH10
A17	0.03	24.00	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH11
A18	0.05	27.08	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH12
A19	0.04	24.40	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH23
A2	1.34	190.64	4.00	1.0000 SCS_Type_II_133.1mm_100yr J7
A20	0.21	136.66	75.00	1.0000 SCS_Type_II_133.1mm_100yr J2
A21	0.17	25.46	18.00	2.0000 SCS_Type_II_133.1mm_100yr SWMF
A22	0.15	20.00	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCB01
A23	0.02	16.00	80.00	2.0000 SCS_Type_II_133.1mm_100yr DCBMH02
A24	0.03	12.00	100.00	2.0000 SCS_Type_II_133.1mm_100yr DCBMH03
A25	0.04	10.00	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH05
A26	0.05	10.00	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH06
A27	0.04	10.00	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH07
A3	0.23	39.72	95.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH24
A4	0.16	51.73	95.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH25
A5	0.11	43.98	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH04
A6	0.12	40.62	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCB17
A7	0.04	19.00	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH18
A8	0.04	24.30	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH19
A 9	0.05	15.48	100.00	1.0000 SCS_Type_II_133.1mm_100yr DCBMH20

Name	Туре	Invert Elev.	Max. Depth	Ponded Area	External Inflow
DCB01	JUNCTION	185.43	1.41	0.0	
DCB09	JUNCTION	185.35	1.41	0.0	
DCB13	JUNCTION	185.43	1.43	0.0	
DCB17	JUNCTION	185.54	1.68	0.0	

DCBMH02	JUNCTION	185.39	1.17	0.0
DCBMH03	JUNCTION	185.29	1.56	0.0
DCBMH04	JUNCTION	185.24	1.40	0.0
DCBMH05	JUNCTION	185.09	1.64	0.0
DCBMH06	JUNCTION	184.84	1.88	0.0
DCBMH07	JUNCTION	184.74	2.00	0.0
DCBMH10	JUNCTION	185.21	1.24	0.0
DCBMH11	JUNCTION	185.02	1.23	0.0
DCBMH12	JUNCTION	184.87	1.25	0.0
DCBMH14	JUNCTION	185.30	1.24	0.0
DCBMH15	JUNCTION	185.19	1.11	0.0
DCBMH16	JUNCTION	185.03	1.09	0.0
DCBMH18	JUNCTION	185.41	1.35	0.0
DCBMH19	JUNCTION	185.17	1.55	0.0
DCBMH20	JUNCTION	185.03	1.61	0.0
DCBMH22	JUNCTION	184.79	1.15	0.0
DCBMH23	JUNCTION	184.62	1.29	0.0
DCBMH24	JUNCTION	184.96	1.20	0.0
DCBMH25	JUNCTION	184.74	1.22	0.0
J11	JUNCTION	185.76	0.24	0.0
J12	JUNCTION	186.00	0.40	0.0
J2	JUNCTION	186.50	0.41	0.0
J7	JUNCTION	182.52	2.08	0.0
MH08	JUNCTION	184.68	2.20	0.0
MH21	JUNCTION	184.89	1.68	0.0
OGS1	JUNCTION	184.48	1.42	0.0
OGS2	JUNCTION	184.58	1.44	0.0
Clark_Street	OUTFALL	185.75	0.00	0.0
HWY26	OUTFALL	182.51	1.52	0.0
SWMF	STORAGE	184.00	1.90	0.0

Name	From Node	To Node	Туре	Length	%Slope Roughness		
C1	DCBMH24	DCBMH25	CONDUIT	27.3	0.5124	0.0130	
C1_2	J7	HWY26	CONDUIT	21.6	0.0464	0.0130	
C10	DCBMH23	OGS1	CONDUIT	8.7	0.4584	0.0130	
C11	J11	Clark Street	CONDUIT	5.8	0.1718	0.0100	

C13	OGS1	SWMF	CONDUIT	10.9	0.2750	0.0130
C14	DCB13	DCBMH14	CONDUIT	27.8	0.4678	0.0130
C15	DCBMH15	DCBMH16	CONDUIT	15.9	0.5017	0.0130
C16	DCBMH14	DCBMH15	CONDUIT	22.1	0.4970	0.0100
C17	DCBMH16	DCBMH22	CONDUIT	33.1	0.4835	0.0130
C18	DCB09	DCBMH10	CONDUIT	28.2	0.4966	0.0130
C19	DCBMH10	DCBMH11	CONDUIT	21.9	0.5013	0.0130
C2	DCBMH25	OGS2	CONDUIT	3.4	0.2962	0.0130
C20	DCBMH11	DCBMH12	CONDUIT	15.9	0.5038	0.0130
C21	DCBMH12	DCBMH23	CONDUIT	33.2	0.5114	0.0130
C22	DCB01	DCBMH02	CONDUIT	12.1	0.3314	0.0130
C22_1	J2	J12	CONDUIT	115.2	0.4339	0.0300
C22_4	J12	SWMF	CONDUIT	16.6	0.6035	0.0300
C23	DCBMH02	DCBMH03	CONDUIT	34.1	0.2934	0.0130
C24	DCBMH03	DCBMH04	CONDUIT	17.4	0.2881	0.0130
C25	DCBMH04	DCBMH05	CONDUIT	26.4	0.3035	0.0130
C26	DCBMH05	DCBMH06	CONDUIT	31.4	0.3185	0.0130
C27	DCBMH06	DCBMH07	CONDUIT	34.2	0.2927	0.0130
C28	DCBMH07	MH08	CONDUIT	20.3	0.2954	0.0130
C29	MH08	OGS1	CONDUIT	14.8	0.3376	0.0130
C3	OGS2	SWMF	CONDUIT	25.5	0.5103	0.0130
C5	DCB17	DCBMH18	CONDUIT	25.8	0.5033	0.0130
C6	DCBMH18	DCBMH19	CONDUIT	32.0	0.4995	0.0130
C7_1	DCBMH19	DCBMH20	CONDUIT	28.9	0.4850	0.0130
C7 2	DCBMH20	MH21	CONDUIT	29.0	0.4831	0.0130
C8	MH21	DCBMH22	CONDUIT	19.3	0.5194	0.0130
C9	DCBMH22	DCBMH23	CONDUIT	18.1	0.4970	0.0130
C12	SWMF	J7	OUTLET			

Conduit	Shape	Full Depth	Full Area	Hyd. Rad.	Max. Width	No. of Barrels	Full Flow
C1	CIRCULAR	0.45	0.16	0.11	0.45	1	0.20
C1_2	RECT CLOSED	1.52	3.71	0.47	2.44	1	3.71
C10	CIRCULAR	0.60	0.28	0.15	0.60	1	0.42
C11	DUMMY	0.00	0.00	0.00	0.00	1	0.00
C13	CIRCULAR	0.75	0.44	0.19	0.75	1	0.58

C14	CIRCULAR	0.30	0.07	0.07	0.30	1	0.07
C15	CIRCULAR	0.30	0.07	0.07	0.30	1	0.07
C16	CIRCULAR	0.30	0.07	0.07	0.30	1	0.09
C17	CIRCULAR	0.38	0.11	0.09	0.38	1	0.12
C18	CIRCULAR	0.30	0.07	0.07	0.30	1	0.07
C19	CIRCULAR	0.30	0.07	0.07	0.30	1	0.07
C2	CIRCULAR	0.60	0.28	0.15	0.60	1	0.33
C20	CIRCULAR	0.38	0.11	0.09	0.38	1	0.12
C21	CIRCULAR	0.45	0.16	0.11	0.45	1	0.20
C22	CIRCULAR	0.38	0.11	0.09	0.38	1	0.10
C22_1	TRAPEZOIDAL	0.40	0.58	0.21	2.65	1	0.45
C22_4	TRAPEZOIDAL	0.40	0.58	0.21	2.65	1	0.53
C23	CIRCULAR	0.38	0.11	0.09	0.38	1	0.09
C24	CIRCULAR	0.38	0.11	0.09	0.38	1	0.09
C25	CIRCULAR	0.38	0.11	0.09	0.38	1	0.10
C26	CIRCULAR	0.45	0.16	0.11	0.45	1	0.16
C27	CIRCULAR	0.60	0.28	0.15	0.60	1	0.33
C28	CIRCULAR	0.60	0.28	0.15	0.60	1	0.33
C29	CIRCULAR	0.60	0.28	0.15	0.60	1	0.36
C3	CIRCULAR	0.75	0.44	0.19	0.75	1	0.80
C5	CIRCULAR	0.38	0.11	0.09	0.38	1	0.12
C6	CIRCULAR	0.38	0.11	0.09	0.38	1	0.12
C7_1	CIRCULAR	0.45	0.16	0.11	0.45	1	0.20
C7_2	CIRCULAR	0.45	0.16	0.11	0.45	1	0.20
C8	CIRCULAR	0.45	0.16	0.11	0.45	1	0.21
C9	CIRCULAR	0.45	0.16	0.11	0.45	1	0.20

\* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*

Analysis Options \*\*\*\*\*\*\*\*

Flow Units ..... CMS

Process Models:

Rainfall/Runoff YES
RDII NO
Snowmelt NO
Groundwater NO
Flow Routing YES
Ponding Allowed YES
Water Quality NO

Infiltration Method ..... GREEN\_AMPT

Flow Routing Method	DYNWAVE	
Surcharge Method	EXTRAN	
Starting Date	05/09/2022	00:00:00
Ending Date	05/11/2022	00:00:00
Antecedent Dry Days	0.0	
Report Time Step	00:01:00	
Wet Time Step	00:05:00	
Dry Time Step	00:05:00	
Routing Time Step	5.00 sec	
Variable Time Step	YES	
Maximum Trials	8	
Number of Threads	4	
Head Tolerance	0 001500 m	

******	Volume	Depth
Runoff Quantity Continuity	hectare-m	mm
* * * * * * * * * * * * * * * * * * * *		
Total Precipitation	0.499	133.100
Evaporation Loss	0.000	0.000
Infiltration Loss	0.112	29.931
Surface Runoff	0.384	102.452
Final Storage	0.003	0.917
Continuity Error (%)	-0.150	
******	** 3	
	Volume	Volume
Flow Routing Continuity	hectare-m	10^6 ltr
Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	0.384	3.841
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	0.384	3.844
Flooding Loss	0.000	0.000
Evaporation Loss	0.000	0.000
Exfiltration Loss	0.000	0.000
Initial Stored Volume	0.000	0.000
Final Stored Volume	0.000	0.000

-0.084

Continuity Error (%) .....

```
*******
Time-Step Critical Elements
******
Link C2 (62.44%)
*****
Highest Flow Instability Indexes
*****
All links are stable.
*******
Most Frequent Nonconverging Nodes
*******
Node Clark Street (0.14%)
Node HWY26 (0.14%)
Node DCBMH20 (0.03%)
Node DCBMH07 (0.03%)
Node DCBMH16 (0.03%)
******
Routing Time Step Summary
*******
                          0.50 sec
Minimum Time Step
Average Time Step
                          3.49 sec
Maximum Time Step
                          5.00 sec
% of Time in Steady State
                          0.00
Average Iterations per Step:
                          2.01
% of Steps Not Converging :
                          0.14
Time Step Frequencies
   5.000 - 3.155 sec
                         52.07 %
   3.155 - 1.991 sec
                         36.21 %
   1.991 - 1.256 sec
                          7.81 %
   1.256 - 0.792 sec
                          2.77 %
```

0.792 - 0.500 sec

1.13 %

Subcatchment	Total Precip mm	Total Runon mm	Total Evap mm	Total Infil mm	Imperv Runoff mm	Perv Runoff mm	Total Runoff mm	Total Runoff 10^6 ltr	Peak Runoff CMS	Runoff Coeff
A1	133.10	0.00	0.00	52.46	1.97	78.96	80.93	0.43	0.22	0.608
A10	133.10	0.00	0.00	0.00	131.28	0.00	131.28	0.04	0.01	0.986
A11	133.10	0.00	0.00	0.00	131.25	0.00	131.25	0.06	0.02	0.986
A12	133.10	0.00	0.00	0.00	131.25	0.00	131.25	0.05	0.02	0.986
A13	133.10	0.00	0.00	0.00	131.25	0.00	131.25	0.04	0.02	0.986
A14	133.10	0.00	0.00	0.00	131.31	0.00	131.31	0.07	0.03	0.987
A15	133.10	0.00	0.00	0.00	131.29	0.00	131.29	0.06	0.02	0.986
A16	133.10	0.00	0.00	0.00	131.27	0.00	131.27	0.05	0.02	0.986
A17	133.10	0.00	0.00	0.00	131.25	0.00	131.25	0.04	0.02	0.986
A18	133.10	0.00	0.00	0.00	131.32	0.00	131.32	0.07	0.03	0.987
A19	133.10	0.00	0.00	0.00	131.30	0.00	131.30	0.06	0.02	0.986
A2	133.10	0.00	0.00	54.53	5.24	78.56	78.56	1.05	0.26	0.590
A20	133.10	0.00	0.00	12.34	98.41	21.03	119.44	0.25	0.11	0.897
A21	133.10	0.00	0.00	42.75	23.61	66.51	90.12	0.16	0.06	0.677
A22	133.10	0.00	0.00	0.00	131.49	0.00	131.49	0.19	0.07	0.988
A23	133.10	0.00	0.00	9.86	104.92	16.81	121.73	0.03	0.01	0.915
A24	133.10	0.00	0.00	0.00	131.30	0.00	131.30	0.04	0.02	0.986
A25	133.10	0.00	0.00	0.00	131.46	0.00	131.46	0.05	0.02	0.988
A26	133.10	0.00	0.00	0.00	131.48	0.00	131.48	0.06	0.02	0.988
A27	133.10	0.00	0.00	0.00	131.47	0.00	131.47	0.06	0.02	0.988
A3	133.10	0.00	0.00	2.47	124.91	4.21	129.12	0.30	0.11	0.970
A4	133.10	0.00	0.00	2.47	124.83	4.20	129.03	0.20	0.08	0.969
A5	133.10	0.00	0.00	0.00	131.37	0.00	131.37	0.14	0.05	0.987
A6	133.10	0.00	0.00	0.00	131.40	0.00	131.40	0.16	0.06	0.987
A7	133.10	0.00	0.00	0.00	131.31	0.00	131.31	0.05	0.02	0.987
A8	133.10	0.00	0.00	0.00	131.30	0.00	131.30	0.06	0.02	0.986
A9	133.10	0.00	0.00	0.00	131.42	0.00	131.42	0.07	0.03	0.987

		Average	Maximum	Maximum	Time	of Max	Reported
		Depth	Depth	HGL	Occu	rrence	Max Depth
Node	Туре	Meters	Meters	Meters	days	hr:min	Meters
DCB01	JUNCTION	0.03	0.65	186.08	0	11:53	0.46
DCB09	JUNCTION	0.01	0.20	185.55	0	12:00	0.20
DCB13	JUNCTION	0.01	0.23	185.66	0	11:55	0.22
DCB17	JUNCTION	0.02	0.19	185.73	0	11:53	0.18
DCBMH02	JUNCTION	0.03	0.65	186.04	0	11:53	0.47
DCBMH03	JUNCTION	0.03	0.59	185.88	0	11:53	0.47
DCBMH04	JUNCTION	0.04	0.53	185.77	0	11:53	0.46
DCBMH05	JUNCTION	0.04	0.49	185.58	0	12:00	0.49
DCBMH06	JUNCTION	0.05	0.69	185.53	0	12:00	0.69
DCBMH07	JUNCTION	0.07	0.78	185.52	0	12:01	0.78
DCBMH10	JUNCTION	0.02	0.33	185.54	0	12:00	0.33
DCBMH11	JUNCTION	0.03	0.50	185.52	0	12:00	0.50
DCBMH12	JUNCTION	0.04	0.65	185.52	0	12:00	0.65
DCBMH14	JUNCTION	0.02	0.35	185.65	0	11:54	0.34
DCBMH15	JUNCTION	0.02	0.63	185.82	0	11:53	0.43
DCBMH16	JUNCTION	0.03	0.70	185.73	0	11:53	0.56
DCBMH18	JUNCTION	0.02	0.22	185.63	0	11:55	0.22
DCBMH19	JUNCTION	0.03	0.44	185.61	0	12:00	0.43
DCBMH20	JUNCTION	0.03	0.56	185.59	0	11:57	0.56
DCBMH22	JUNCTION	0.06	0.76	185.55	0	12:00	0.76
DCBMH23	JUNCTION	0.12	0.89	185.51	0	12:01	0.89
DCBMH24	JUNCTION	0.04	0.55	185.51	0	12:01	0.55
DCBMH25	JUNCTION	0.07	0.76	185.50	0	12:02	0.76
J11	JUNCTION	0.00	0.00	185.76	0	00:00	0.00
J12	JUNCTION	0.03	0.20	186.20	0	11:56	0.20
J2	JUNCTION	0.02	0.23	186.73	0	11:54	0.23
J7	JUNCTION	0.04	0.25	182.77	0	12:01	0.25
MH08	JUNCTION	0.10	0.83	185.51	0	12:02	0.83
MH21	JUNCTION	0.04	0.68	185.57	0	12:00	0.68
OGS1	JUNCTION	0.19	1.02	185.50	0	12:02	1.02
OGS2	JUNCTION	0.13	0.92	185.50	0	12:02	0.92
Clark_Street	OUTFALL	0.00	0.00	185.75	0	00:00	0.00
HWY26	OUTFALL	0.02	0.20	182.71	0	12:01	0.20
SWMF	STORAGE	0.49	1.50	185.50	0	12:03	1.50

Node Type	Maximum Lateral Inflow CMS	Maximum Total Inflow CMS	0ccu	of Max arrence hr:min	Lateral Inflow Volume 10^6 ltr	Total Inflow Volume 10^6 ltr	Flow Balance Error Percent
DCB01 JUNCTION	0.072	0.072	0	11:54	0.194	0.194	0.004
DCB09 JUNCTION	0.023	0.023	0	11:54	0.0595	0.0595	-0.027
DCB13 JUNCTION		0.024	0	11:54	0.0632	0.0632	-0.014
DCB17 JUNCTION	0.060	0.060	0	11:54	0.156	0.156	0.007
DCBMH02 JUNCTION	0.011	0.084	0	11:54	0.0255	0.219	-0.010
DCBMH03 JUNCTION	0.015	0.101	0	11:54	0.0401	0.259	-0.019
DCBMH04 JUNCTION	0.055	0.156	0	11:54	0.142	0.402	0.305
DCBMH05 JUNCTION	0.021	0.177	0	11:54	0.0545	0.455	0.088
DCBMH06 JUNCTION	0.024	0.192	0	11:54	0.0638	0.518	-0.337
DCBMH07 JUNCTION	0.022	0.216	0	11:54	0.0587	0.579	-0.024
DCBMH10 JUNCTION	0.021	0.044	0	11:54	0.0549	0.114	0.302
DCBMH11 JUNCTION	0.017	0.059	0	11:53	0.0431	0.157	-0.117
DCBMH12 JUNCTION	0.027	0.085	0	11:53	0.0698	0.227	-0.090
DCBMH14 JUNCTION	0.020	0.045	0	11:53	0.0531	0.116	-0.011
DCBMH15 JUNCTION	0.016	0.058	0	11:52	0.0427	0.159	0.133
DCBMH16 JUNCTION	0.027	0.080	0	11:55	0.0696	0.228	0.013
DCBMH18 JUNCTION	0.018	0.078	0	11:53	0.0464	0.203	0.393
DCBMH19 JUNCTION	0.022	0.100	0	11:54	0.0564	0.258	-0.316
DCBMH20 JUNCTION	0.025	0.118	0	11:54	0.0659	0.325	-0.035
DCBMH22 JUNCTION	0.014	0.210	0	11:54	0.0351	0.589	-0.082
DCBMH23 JUNCTION	0.022	0.309	0	11:54	0.0565	0.873	-0.051
DCBMH24 JUNCTION	0.115	0.115	0	11:54	0.298	0.298	0.082
DCBMH25 JUNCTION	0.078	0.190	0	11:54	0.2	0.498	-0.162
J11 JUNCTION	0.224	0.224	0	11:54	0.43	0.43	0.000
J12 JUNCTION	0.000	0.103	0	11:55	0	0.251	0.179
J2 JUNCTION	0.106	0.106	0	11:54	0.251	0.251	-0.170
J7 JUNCTION	0.256	0.692	0	12:01	1.05	3.41	0.000
MH08 JUNCTION	0.000	0.214	0	11:54	0	0.579	-0.014
MH21 JUNCTION	0.000	0.117	0	11:54	0	0.325	-0.045
OGS1 JUNCTION	0.000	0.522	0	11:54	0	1.45	-0.043

OGS2	JUNCTION	0.000	0.187	0	11:54	0	0.499	-0.119
Clark_Street	OUTFALL	0.000	0.224	0	11:54	0	0.43	0.000
HWY26	OUTFALL	0.000	0.692	0	12:01	0	3.41	0.000
SWMF	STORAGE	0.056	0.850	0	11:54	0.158	2.36	-0.000

Surcharging occurs when water rises above the top of the highest conduit.

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Node	Туре		Max. Height Above Crown Meters	Below Rim
DCB01	JUNCTION	0.05	0.274	0.761
DCBMH02	JUNCTION	0.07	0.272	0.523
DCBMH03	JUNCTION	0.14	0.217	0.968
DCBMH04	JUNCTION	0.15	0.157	0.868
DCBMH05	JUNCTION	0.12	0.044	1.146
DCBMH06	JUNCTION	0.22	0.088	1.192
DCBMH07	JUNCTION	0.38	0.176	1.224
DCBMH10	JUNCTION	0.09	0.033	0.907
DCBMH11	JUNCTION	0.29	0.123	0.727
DCBMH12	JUNCTION	0.42	0.196	0.604
DCBMH14	JUNCTION	0.11	0.054	0.886
DCBMH15	JUNCTION	0.20	0.333	0.477
DCBMH16	JUNCTION	0.31	0.317	0.393
DCBMH20	JUNCTION	0.20	0.113	1.052
DCBMH22	JUNCTION	0.57	0.307	0.388
DCBMH23	JUNCTION	0.61	0.290	0.400
DCBMH24	JUNCTION	0.26	0.099	0.651
DCBMH25	JUNCTION	0.36	0.160	0.460
J11	JUNCTION	48.00	0.000	0.240
MH08	JUNCTION	0.48	0.229	1.371
MH21	JUNCTION	0.41	0.228	1.002
OGS1	JUNCTION	0.58	0.275	0.395
OGS2	JUNCTION	0.37	0.169	0.521

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No nodes were flooded.

	Average	Avg	Evap	Exfil	Maximum	Max	Time of Max	Maximum
	Volume	Pcnt	Pcnt	Pcnt	Volume	Pcnt	Occurrence	Outflow
Storage Unit	1000 m³	Full	Loss	Loss	1000 m³	Full	days hr:min	CMS
SWMF	0.177	14.8	0.0	0.0	0.798	66.9	0 12:03	0.450

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	Flow Freq	Avg Flow	Max Flow	Total Volume
Outfall Node	Pcnt	CMS	CMS	10^6 ltr
Clark_Street	63.01 91.88	0.009	0.224	0.430
System	77.45	0.048	0.876	3.844

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Maximum Time of Max Maximum Max/ Max/ |Flow| Occurrence |Veloc| Full Full

Link	Туре	CMS	days	hr:min	m/sec	Flow	Depth
C1	CONDUIT	0.112	0	11:54	0.90	0.55	1.00
C1_2	CONDUIT	0.692	0	12:01	1.26	0.19	0.15
C10	CONDUIT	0.310	0	11:54	1.09	0.74	1.00
C11	DUMMY			11:54			
C13	CONDUIT	0.522	0	11:54	1.18	0.89	1.00
C14	CONDUIT			11:53	0.75	0.37	0.88
C15	CONDUIT	0.056	0	11:55	1.10	0.82	1.00
C16	CONDUIT	0.042	0	11:52	0.93	0.47	1.00
C17	CONDUIT			11:55	0.91	0.66	1.00
C18	CONDUIT	0.023	0	11:54	0.66	0.34	0.83
C19	CONDUIT	0.042	0	11:52	1.01	0.61	1.00
C2	CONDUIT		0	11:54	0.69	0.56	1.00
C20	CONDUIT	0.058	0	11:53	0.92	0.46	1.00
C21	CONDUIT	0.084	0	11:53	0.54	0.41	1.00
C22	CONDUIT			11:54	0.82	0.73	1.00
C22_1	CONDUIT	0.103	0	11:55	0.54	0.23	0.53
C22_4	CONDUIT		0	11:56	0.72	0.19	0.44
C23	CONDUIT	0.085	0	11:54	0.77	0.90	1.00
C24	CONDUIT	0.101	0	11:54	0.92	1.07	1.00
C25	CONDUIT	0.156	0	11:54	1.46	1.62	1.00
C26	CONDUIT			11:54		1.05	1.00
C27	CONDUIT			11:54	0.69	0.58	1.00
C28	CONDUIT		0	11:54	0.76	0.64	1.00
C29	CONDUIT	0.215	0	11:54	0.76	0.60	1.00
C3	CONDUIT	0.185	0	11:54	0.50	0.23	1.00
C5	CONDUIT	0.060	0	11:53	1.01	0.48	0.53
C6	CONDUIT			11:54	1.20	0.63	0.75
C7_1	CONDUIT			11:55	1.02	0.47	0.98
C7_2	CONDUIT			11:54	1.03	0.59	1.00
C8	CONDUIT	0.117	0	11:54	0.74	0.57	
C9	CONDUIT	0.210	0	11:54	1.32	1.05	1.00
C12	DUMMY	0.450	0	12:03			

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Flow Classification Summary

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	Adjusted			Fract	ion of	Time	in Flo	w Clas	s	
	/Actual		Up	Down	Sub	Sup	Up	Down	Norm	Inlet
Conduit	Length	Dry	Dry	Dry	Crit	Crit	Crit	Crit	Ltd	Ctrl
C1	1.00	0.03	0.00	0.00	0.09	0.00	0.00	0.88	0.06	0.00
C1 2	1.00	0.03	0.00	0.00	0.97	0.00	0.00	0.00	0.00	0.00
C10	1.00	0.03	0.00	0.00	0.33	0.00	0.00	0.64	0.01	0.00
C13	1.00	0.03	0.00	0.00	0.39	0.00	0.00	0.58	0.01	0.00
C14	1.00	0.03	0.00	0.00	0.97	0.00	0.00	0.00	0.52	0.00
C15	1.00	0.03	0.00	0.00	0.02	0.00	0.00	0.96	0.00	0.00
C16	1.00	0.03	0.00	0.00	0.97	0.00	0.00	0.00	0.96	0.00
C17	1.00	0.03	0.00	0.00	0.05	0.00	0.00	0.92	0.03	0.00
C18	1.00	0.03	0.00	0.00	0.97	0.00	0.00	0.00	0.96	0.00
C19	1.00	0.03	0.00	0.00	0.02	0.00	0.00	0.95	0.01	0.00
C2	1.00	0.03	0.00	0.00	0.23	0.00	0.00	0.74	0.00	0.00
C20	1.00	0.03	0.00	0.00	0.03	0.00	0.00	0.94	0.01	0.00
C21	1.00	0.03	0.00	0.00	0.28	0.00	0.00	0.69	0.23	0.00
C22	1.00	0.03	0.00	0.00	0.97	0.00	0.00	0.00	0.95	0.00
C22_1	1.00	0.03	0.00	0.00	0.97	0.00	0.00	0.00	0.84	0.00
C22_4	1.00	0.03	0.00	0.00	0.00	0.00	0.00	0.97	0.00	0.00
C23	1.00	0.03	0.00	0.00	0.97	0.00	0.00	0.00	0.68	0.00
C24	1.00	0.03	0.00	0.00	0.97	0.00	0.00	0.00	0.96	0.00
C25	1.00	0.03	0.00	0.00	0.01	0.00	0.00	0.96	0.00	0.00
C26	1.00	0.03	0.00	0.00	0.02	0.00	0.00	0.95	0.01	0.00
C27	1.00	0.03	0.00	0.00	0.97	0.00	0.00	0.00	0.65	0.00
C28	1.00	0.03	0.00	0.00	0.97	0.00	0.00	0.00	0.56	0.00
C29	1.00	0.03	0.00	0.00	0.31	0.00	0.00	0.66	0.02	0.00
C3	1.00	0.03	0.00	0.00	0.40	0.00	0.00	0.57	0.07	0.00
C5	1.00	0.03	0.00	0.00	0.97	0.00	0.00	0.00	0.96	0.00
C6	1.00	0.03	0.00	0.00	0.01	0.00	0.00	0.96	0.01	0.00
C7_1	1.00	0.03	0.00	0.00	0.97	0.00	0.00	0.00	0.95	0.00
C7_2	1.00	0.03	0.00	0.00	0.97	0.00	0.00	0.00	0.14	0.00
C8	1.00	0.03	0.02	0.00	0.95	0.00	0.00	0.00	0.93	0.00
C9	1.00	0.03	0.00	0.00	0.28	0.00	0.00	0.69	0.15	0.00

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Conduit Surcharge Summary

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Conduit	Both Ends	Upstream	Dnstream	Hours Above Full Normal Flow	Capacity Limited
C1				0.01	
C10	0.61	0.61	0.70	0.01	0.01
C13	0.58	0.58	0.64	0.01	0.01
C14	0.01	0.01	0.11	0.01	0.01
C15	0.19	0.19	0.31	0.01	0.01
C16	0.11	0.11	0.20	0.01	0.01
C17	0.32	0.32	0.57	0.01	0.01
C18	0.01	0.01	0.09	0.01	0.01
C19	0.09	0.09	0.29	0.01	0.01
C2	0.36	0.36	0.37	0.01	0.01
C20	0.30	0.30	0.43	0.01	0.01
C21	0.42	0.42	0.79	0.01	0.01
C22	0.05	0.05	0.07	0.01	0.01
C23	0.07	0.07	0.14	0.01	0.01
C24	0.14	0.14	0.15	0.02	0.01
C25	0.11	0.15	0.13	0.19	0.09
C26	0.12	0.12	0.22	0.02	0.01
C27	0.22	0.22	0.38	0.01	0.01
C28	0.38	0.38	0.48	0.01	0.01
C29	0.48	0.48	0.58	0.01	0.01
C3	0.37	0.37	0.64	0.01	0.01
C7_1	0.01	0.01	0.20	0.01	0.01
C7_2	0.20	0.20	0.41	0.01	0.01
C8	0.41	0.41	0.58	0.01	0.01
C9	0.58	0.58	0.79	0.03	0.03

Analysis begun on: Wed Dec 13 10:55:17 2023 Analysis ended on: Wed Dec 13 10:55:18 2023

Total elapsed time: 00:00:01

# Profile 1 DCB17 to Hwy 26 Outlet

						100,1 000 010					
	HGL										Peak values
Links:	C1_2 Q=0.692 m³/s L=21.555 m D=1.52 m V=1.264 m/s S=0.00046 m/m	C12 Q=0.45 m³/s V=0 m/s	C13 Q=0.515 m³/s L=10.911 m D=0.75 m V=1.166 m/s S=0.00275 m/m	C10 Q=0.307 m <sup>3</sup> /s L=8.727 m D=0.6 m V=1.087 m/s S=0.00458 m/m	C9 Q=0.209 m <sup>3</sup> /s L=18.108 m D=0.45 m V=1.313 m/s S=0.00497 m/n	C8 Q=0.117 m L=19.254 m D=0.45 m V=0.736 m/ n S=0.00519	L=28.9 D=0.45 S V=1.03	17 m³/s G 98 m L 5 m E 34 m/s V	27_1 2=0.093 m³/s =28.865 m )=0.45 m /=1.024 m/s 6=0.00485 m/m	C6 Q=0.078 m <sup>3</sup> /s L=32.03 m D=0.375 m V=1.187 m/s S=0.005 m/m	C5 Q=0.06 m³/s L=25.828 m D=0.375 m V=1.006 m/s S=0.00503 m/m
											187
											186.5
											185.5
											185
											184.5
											183.5
			40				100	110	400	100	183
0 Nodes	20 : HWY26 H=182.7115 m M=182.7115 m R=184.6 m	J7 H=182.7669 m M=182.7669 m R=184.6 m	SWMF H=185.4994 m M=185.4994 m R=186.3 m	OGS1 H=185.5033 m M=185.5033 m R=185.9 m	B0 DCBMH23 H=185.5089 m M=185.5089 m R=185.91 m	100 DCBMH22 H=185.5504 m M=185.5504 m R=185.94 m	120 MH21 H=185.5661 m M=185.5661 m R=186.57 m	140 DCBMH20 H=185.5902 m M=185.5902 m R=186.645 m	160 DCBMH19 H=185.6026 M=185.6026 R=186.72 m	m M=185.6261 m	DCB17 H=185.7248 m M=185.7248 m R=187.22 m

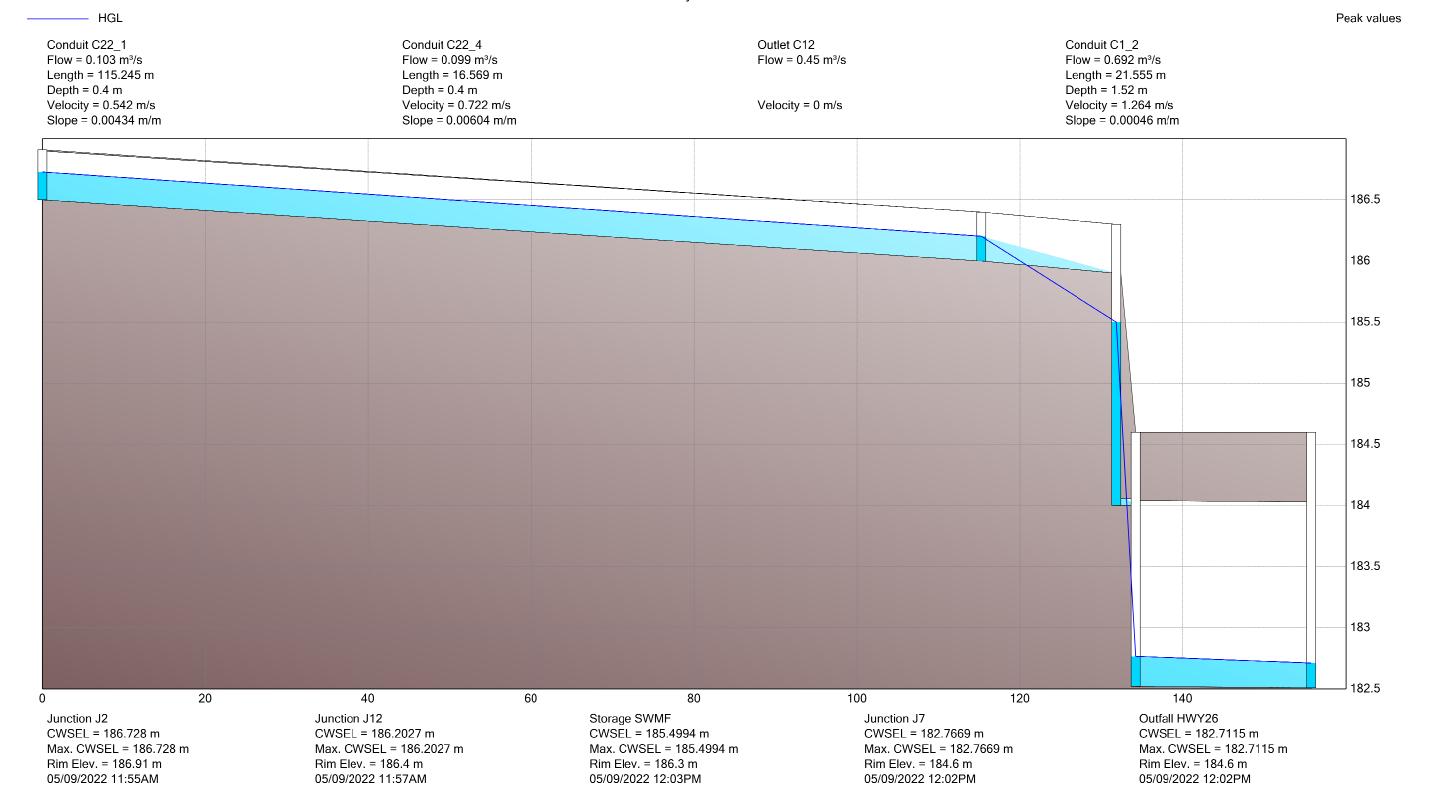
# Profile 2 DCB13 to Hwy 26 Outlet

HGL								
								Peak v
S: C14 Q=0.024 m³/s L=27.79 m D=0.3 m V=0.734 m/s S=0.00468 m/m	C16 Q=0.04 m³/s L=22.135 m D=0.3 m V=0.931 m/s S=0.00497 m/m	C15 Q=0.055 m³/s L=15.947 m D=0.3 m V=1.065 m/s S=0.00502 m/m	C17 Q=0.079 m³/s L=33.094 m D=0.375 m V=0.91 m/s S=0.00483 m/m	C9 Q=0.209 m³/s L=18.108 m D=0.45 m V=1.313 m/s S=0.00497 m/m	C10 Q=0.307 m³/s L=8.727 m D=0.6 m V=1.087 m/s S=0.00458 m/m	C13 Q=0.515 m³/s L=10.911 m D=0.75 m V=1.166 m/s S=0.00275 m/m	C12 Q=0.45 m³/s V=0 m/s	C1_2 Q=0.692 m³/s L=21.555 m D=1.52 m V=1.264 m/s S=0.00046 m/m
S=0.00468 m/m	S=0.00497 m/m	S=0.00502 m/m	S=0.00483 m/m	S=0.00497 m/m	S=0.00458 m/m	S=0.00275 m/m	V-0 111/3	S=0.00046 m/m
								186
								186
								185
								185
								184
								184
								100
								183
								183
								183
	20 4(				120 OGS1	140 SWMF	160	183
) :s: DCB13 H=185.653 m M=185.653 m	DCBMH14 [ H=185.6405 m	DCBMH15 DC H=185.6187 m H=	CBMH16 DCBM 185.5884 m H=185		OGS1 m H=185.5033 m	SWMF H=185.4994 m	160 J7 H=182.7669 m M=182.7669 m	183 183 182 HWY26 H=182.7115 m M=182.7115 m

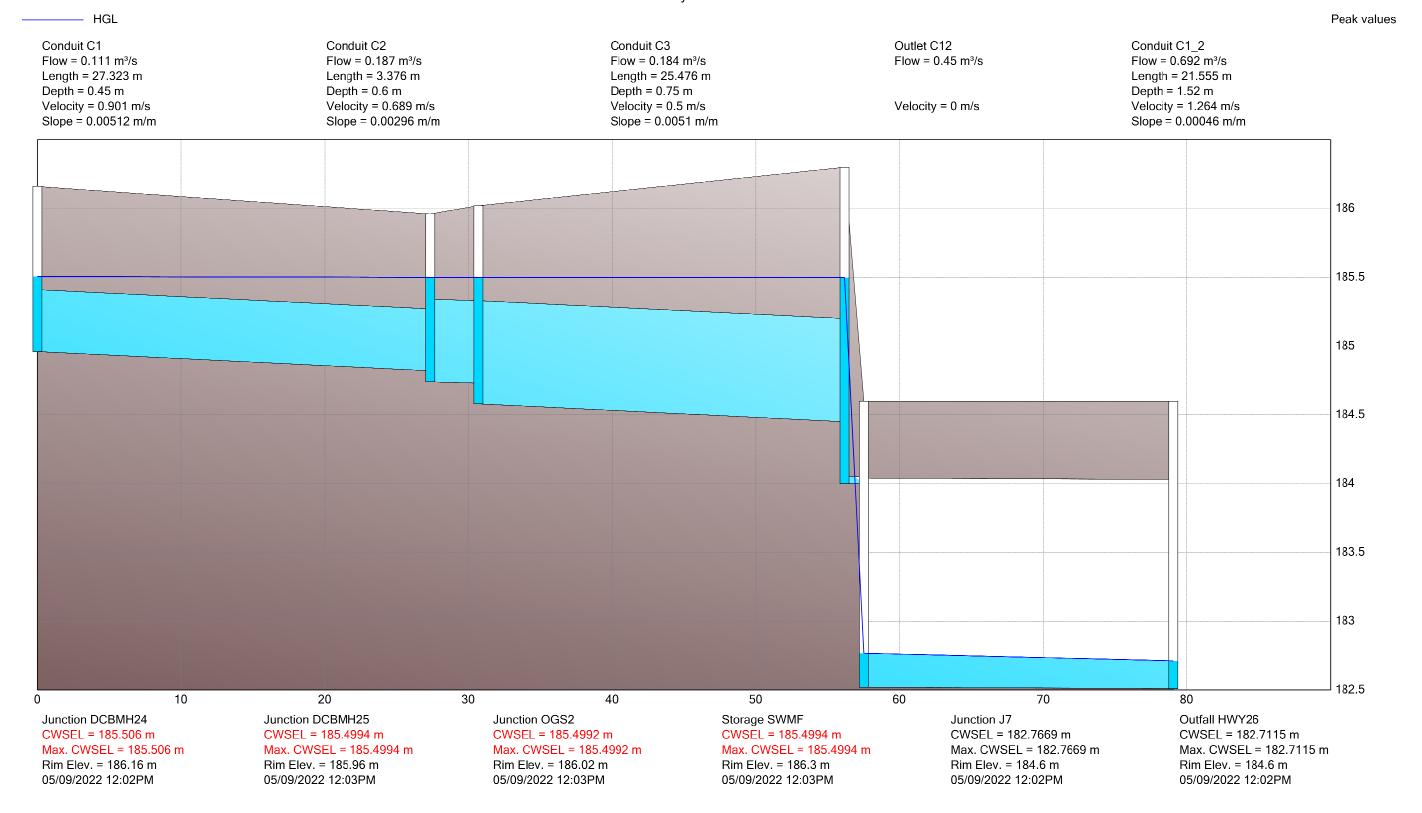
# Profile 3 DCB09 to Hwy 26 Outlet

	—— HGL								Pea	ak values
	C18 Q=0.023 m <sup>3</sup> /s L=28.194 m D=0.3 m V=0.663 m/s S=0.00497 m/m	C19 Q=0.042 m³/s L=21.943 m D=0.3 m V=0.999 m/s S=0.00501 m/m	C20 Q=0.054 m³/s L=15.88 m D=0.375 m V=0.909 m/s S=0.00504 m/m	C21 Q=0.08 L=33.24 D=0.45 V=0.544 S=0.005	l m³/s m m ŀ m/s	C10 Q=0.307 m³/s L=8.727 m D=0.6 m V=1.087 m/s S=0.00458 m/m	C13 Q=0.515 m <sup>3</sup> /s L=10.911 m D=0.75 m V=1.166 m/s S=0.00275 m/m	C12 Q=0.45 m³/s V=0 m/s	C1_2 Q=0.692 m³/s L=21.555 m D=1.52 m V=1.264 m/s S=0.00046 m/m	
			-							186.5
										186
										185.5
										185
										184.5
										184
										183.5
										183
-										
0	20		40	60	80	100	120	140		182.5
Nodes:	DCB09 H=185.5452 m M=185.5452 m R=186.76 m	DCBMH10 H=185.5417 m M=185.5417 m R=186.45 m	DCBMH11 H=185.5219 m M=185.5219 m R=186.25 m	DCBMH12 H=185.5151 m M=185.5151 m R=186.12 m	DCBMH23 H=185.5089 m M=185.5089 m R=185.91 m	OGS1 H=185.5033 m M=185.5033 m R=185.9 m	SWMF H=185.4994 m M=185.4994 m R=186.3 m	J7 H=182.7669 m M=182.7669 m R=184.6 m	HWY26 H=182.7115 m M=182.7115 m R=184.6 m	

# Profile 4 Swale to Hwy 26 Outlet



## Profile 5 DCBMH24 to Hwy 26 Outlet



# Profile 6 DCB01 to Hwy 26 Outlet

						10091 303	Storm					
	——— HGL											Peak values
Links:	C1_2 Q=0.692 m³/s L=21.555 m D=1.52 m V=1.264 m/s S=0.00046 m/m	C12 Q=0.45 m³/s V=0 m/s	C13 Q=0.515 m³/s L=10.911 m D=0.75 m V=1.166 m/s S=0.00275 m/m	C29 Q=0.209 m³/s L=14.809 m D=0.6 m V=0.738 m/s S=0.00338 m/m	C28 Q=0.209 m³/s L=20.311 m D=0.6 m V=0.739 m/s S=0.00295 m/m	C27 Q=0.188 m³/s L=34.163 m D=0.6 m V=0.68 m/s S=0.00293 m	L=31.396 D=0.45 r V=1.269	6 m L=26. m D=0.3 m/s V=1.4	358 m L=1 375 m D=0 463 m/s V=0	0.101 m³/s Q 7.357 m L= 0.375 m D= 0.91 m/s V=	23 =0.085 m³/s =34.083 m =0.375 m =0.767 m/s =0.00293 m/m	C22 Q=0.073 m³/s L=12.071 m D=0.375 m V=0.816 m/s S=0.00331 m/m
												186.5
												186
												185.5
												185
												184.5
-												184
											-	183.5
											-	183
												100 5
0	20	40	60	80	100	120	140	160	180	200	220	182.5
Nodes	: HWY26 H=182.7115 m M=182.7115 m R=184.6 m	J7 H=182.7669 m M=182.7669 m R=184.6 m	SWMF H=185.4994 m M=185.4994 m R=186.3 m	OGS1 H=185.5033 m M=185.5033 m R=185.9 m	MH08 H=185.5085 m M=185.5085 m R=186.88 m	DCBMH07 H=185.5151 m M=185.5151 m R=186.74 m	DCBMH06 H=185.5268 m M=185.5268 m R=186.72 m	DCBMH05 H=185.5824 m M=185.5824 m R=186.73 m	DCBMH04 H=185.7025 m M=185.7025 m R=186.64 m	DCBMH03 H=185.7622 m M=185.7622 m R=186.85 m	DCBMH02 H=185.8565 m M=185.8565 m R=186.56 m	DCB01 H=185.8863 m M=185.8863 m R=186.84 m



# CDS ESTIMATED NET ANNUAL SOLIDS LOAD REDUCTION BASED ON THE RATIONAL RAINFALL METHOD BASED ON A FINE PARTICLE SIZE DISTRIBUTION



Project Name:185 Clark StreetEngineer:Capes EngineeringLocation:Blue Mountains, ONContact:Clayton Capes, P.Eng

OGS #: 1 Report Date: 15-Aug-22

Area0.920haRainfall Station #198Weighted C0.90Particle Size DistributionFINECDS Model2025CDS Treatment Capacity45I/s

Rainfall Intensity <sup>1</sup> (mm/hr)	Percent Rainfall Volume <sup>1</sup>	Cumulative Rainfall Volume	Total Flowrate (I/s)	<u>Treated</u> <u>Flowrate (I/s)</u>	Operating Rate (%)	Removal Efficiency (%)	Incremental Removal (%)
1.0	10.8%	21.1%	2.3	2.3	5.1	97.4	10.5
1.5	10.1%	31.2%	3.5	3.5	7.6	96.7	9.7
2.0	8.5%	39.7%	4.6	4.6	10.2	95.9	8.1
2.5	6.7%	46.4%	5.8	5.8	12.7	95.2	6.4
3.0	6.1%	52.4%	6.9	6.9	15.2	94.5	5.7
3.5	4.1%	56.5%	8.1	8.1	17.8	93.8	3.8
4.0	4.2%	60.7%	9.2	9.2	20.3	93.0	3.9
4.5	3.7%	64.4%	10.4	10.4	22.9	92.3	3.4
5.0	3.9%	68.2%	11.5	11.5	25.4	91.6	3.5
6.0	5.3%	73.5%	13.8	13.8	30.5	90.1	4.8
7.0	3.8%	77.4%	16.1	16.1	35.6	88.7	3.4
8.0	2.8%	80.1%	18.4	18.4	40.6	87.2	2.4
9.0	2.5%	82.6%	20.7	20.7	45.7	85.8	2.1
10.0	2.1%	84.7%	23.0	23.0	50.8	84.3	1.8
15.0	5.7%	90.5%	34.5	34.5	76.2	77.0	4.4
20.0	3.4%	93.8%	46.0	45.3	100.0	69.1	2.3
25.0	3.4%	97.2%	57.5	45.3	100.0	55.3	1.9
30.0	0.8%	98.0%	69.1	45.3	100.0	46.1	0.4
35.0	0.9%	98.8%	80.6	45.3	100.0	39.5	0.3
40.0	0.4%	99.3%	92.1	45.3	100.0	34.5	0.1
45.0	0.5%	99.7%	103.6	45.3	100.0	30.7	0.1
50.0	0.3%	100.0%	115.1	45.3	100.0	27.6	0.1
	•		•				89.4

Removal Efficiency Adjustment<sup>2</sup> =

6.5% **82.9%** 

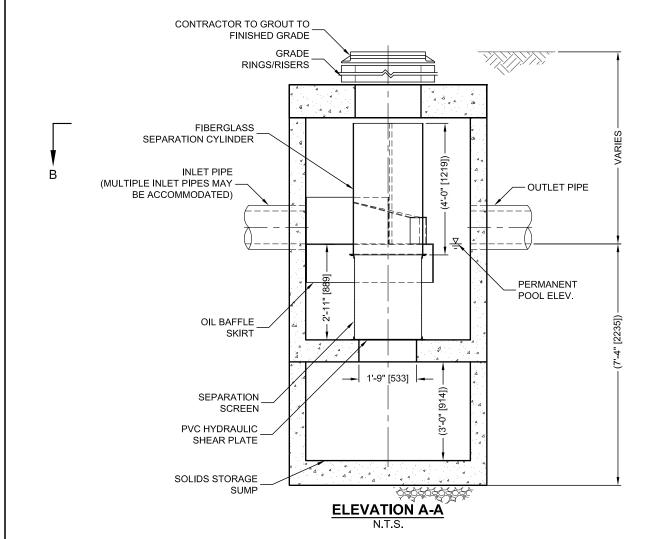
Predicted Net Annual Load Removal Efficiency = Predicted Annual Rainfall Treated =

96.9%

1 - Based on 38 years of hourly rainfall data from Canadian Station 6166132, Owen Sound ON

- 2 Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.
- 3 CDS Efficiency based on testing conducted at the University of Central Florida
- 4 CDS design flowrate and scaling based on standard manufacturer model & product specifications

# PLAN VIEW B-B





#### CDS PMSU2025-5-C DESIGN NOTES

THE STANDARD CDS PMSU2025-5-C CONFIGURATION IS SHOWN. ALTERNATE CONFIGURATIONS ARE AVAILABLE AND ARE LISTED BELOW. SOME CONFIGURATIONS MAY BE COMBINED TO SUIT SITE REQUIREMENTS.

#### **CONFIGURATION DESCRIPTION**

GRATED INLET ONLY (NO INLET PIPE)

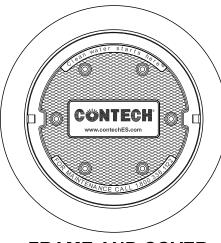
GRATED INLET WITH INLET PIPE OR PIPES

CURB INLET ONLY (NO INLET PIPE)

CURB INLET WITH INLET PIPE OR PIPES

CUSTOMIZABLE SUMP DEPTH AVAILABLE

ANTI-FLOTATION DESIGN AVAILABLE UPON REQUEST



# FRAME AND COVER (DIAMETER VARIES) N.T.S.

SITE SPECIFIC								
DATA REQUIREMENTS								
STRUCTURE ID								
WATER QUALITY	FLOW RAT	Έ (	CFS OR L/s)		*			
PEAK FLOW RAT	E (CFS OR	L/s)	,		*			
RETURN PERIOD	OF PEAK F	LO	W (YRS)		*			
SCREEN APERTU	JRE (2400 C	R 4	1700)		*			
		_						
PIPE DATA:	I.E.		MATERIAL	D	IAMETER			
INLET PIPE 1	*		*		*			
INLET PIPE 2	*		*		*			
OUTLET PIPE	*		*		*			
RIM ELEVATION					*			
ANTI-FLOTATION	BALLAST		WIDTH	T	HEIGHT			
NOTES/SPECIAL REQUIREMENTS:								
* PER ENGINEER	OF RECOF	RD						

#### GENERAL NOTES

- 1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
- 2. DIMENSIONS MARKED WITH () ARE REFERENCE DIMENSIONS. ACTUAL DIMENSIONS MAY VARY.
- 3. FOR FABRICATION DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHTS, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS LLC REPRESENTATIVE. www.contechES.com
- 4. CDS WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING.
- 5. STRUCTURE SHALL MEET AASHTO HS20 AND CASTINGS SHALL MEET HS20 (AASHTO M 306) LOAD RATING, ASSUMING GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION.
- 6. PVC HYDRAULIC SHEAR PLATE IS PLACED ON SHELF AT BOTTOM OF SCREEN CYLINDER. REMOVE AND REPLACE AS NECESSARY DURING MAINTENANCE CLEANING.

#### **INSTALLATION NOTES**

- A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
- B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE CDS MANHOLE STRUCTURE (LIFTING CLUTCHES PROVIDED).
- C. CONTRACTOR TO ADD JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS, AND ASSEMBLE STRUCTURE.
- D. CONTRACTOR TO PROVIDE, INSTALL, AND GROUT PIPES. MATCH PIPE INVERTS WITH ELEVATIONS SHOWN.
- CONTRACTOR TO TAKE APPROPRIATE MEASURES TO ASSURE UNIT IS WATER TIGHT, HOLDING WATER TO FLOWLINE INVERT MINIMUM. IT IS SUGGESTED THAT ALL JOINTS BELOW PIPE INVERTS ARE GROUTED.



800-338-1122 513-645-7000 513-645-7993 FAX

CDS PMSU2025-5-C INLINE CDS STANDARD DETAIL



# CDS ESTIMATED NET ANNUAL SOLIDS LOAD REDUCTION BASED ON THE RATIONAL RAINFALL METHOD BASED ON A FINE PARTICLE SIZE DISTRIBUTION



Project Name:185 Clark StreetEngineer:Capes EngineeringLocation:Blue Mountains, ONContact:Clayton Capes, P.Eng

OGS #: 2 Report Date: 15-Aug-22

Area0.360haRainfall Station #198Weighted C0.90Particle Size DistributionFINECDS Model2015CDS Treatment Capacity20I/s

Rainfall Intensity <sup>1</sup> (mm/hr)	Percent Rainfall Volume <sup>1</sup>	Cumulative Rainfall Volume	Total Flowrate (I/s)	<u>Treated</u> <u>Flowrate (I/s)</u>	Operating Rate (%)	Removal Efficiency (%)	Incremental Removal (%)
1.0	10.8%	21.1%	0.9	0.9	4.5	97.6	10.5
1.5	10.1%	31.2%	1.4	1.4	6.8	96.9	9.8
2.0	8.5%	39.7%	1.8	1.8	9.1	96.3	8.2
2.5	6.7%	46.4%	2.3	2.3	11.4	95.6	6.4
3.0	6.1%	52.4%	2.7	2.7	13.6	94.9	5.7
3.5	4.1%	56.5%	3.2	3.2	15.9	94.3	3.9
4.0	4.2%	60.7%	3.6	3.6	18.2	93.6	3.9
4.5	3.7%	64.4%	4.1	4.1	20.4	93.0	3.4
5.0	3.9%	68.2%	4.5	4.5	22.7	92.3	3.6
6.0	5.3%	73.5%	5.4	5.4	27.3	91.0	4.8
7.0	3.8%	77.4%	6.3	6.3	31.8	89.7	3.4
8.0	2.8%	80.1%	7.2	7.2	36.3	88.4	2.4
9.0	2.5%	82.6%	8.1	8.1	40.9	87.1	2.2
10.0	2.1%	84.7%	9.0	9.0	45.4	85.8	1.8
15.0	5.7%	90.5%	13.5	13.5	68.2	79.3	4.5
20.0	3.4%	93.8%	18.0	18.0	90.9	72.8	2.5
25.0	3.4%	97.2%	22.5	19.8	100.0	61.8	2.1
30.0	0.8%	98.0%	27.0	19.8	100.0	51.5	0.4
35.0	0.9%	98.8%	31.5	19.8	100.0	44.1	0.4
40.0	0.4%	99.3%	36.0	19.8	100.0	38.6	0.2
45.0	0.5%	99.7%	40.5	19.8	100.0	34.3	0.2
50.0	0.3%	100.0%	45.0	19.8	100.0	30.9	0.1
							90.5

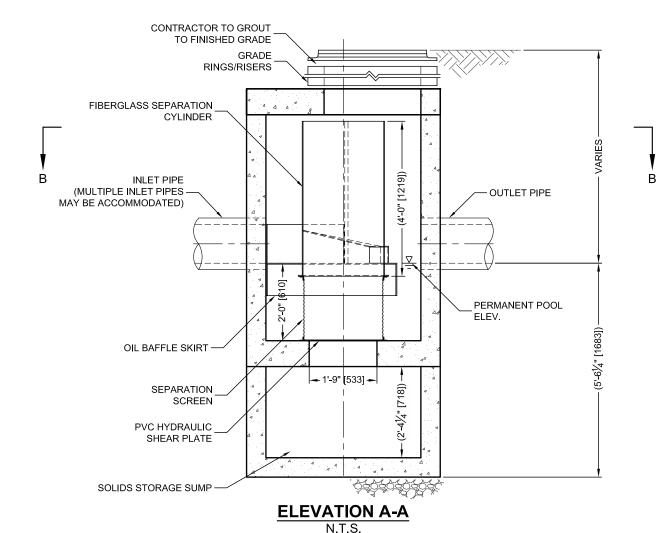
Removal Efficiency Adjustment<sup>2</sup> =

6.5%

Predicted Net Annual Load Removal Efficiency = Predicted Annual Rainfall Treated = 84.0% 97.4%

- 1 Based on 38 years of hourly rainfall data from Canadian Station 6166132, Owen Sound ON
- 2 Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.
- 3 CDS Efficiency based on testing conducted at the University of Central Florida
- 4 CDS design flowrate and scaling based on standard manufacturer model & product specifications

# PLAN VIEW B-B





#### CDS PMSU2015-4-C DESIGN NOTES

THE STANDARD CDS PMSU2015-4-C CONFIGURATION IS SHOWN. ALTERNATE CONFIGURATIONS ARE AVAILABLE AND ARE LISTED BELOW. SOME CONFIGURATIONS MAY BE COMBINED TO SUIT SITE REQUIREMENTS.

#### **CONFIGURATION DESCRIPTION**

GRATED INLET ONLY (NO INLET PIPE)

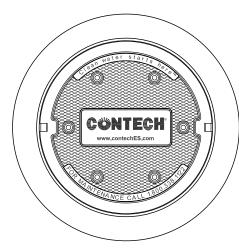
GRATED INLET WITH INLET PIPE OR PIPES

CURB INLET ONLY (NO INLET PIPE)

CURB INLET WITH INLET PIPE OR PIPES

CUSTOMIZABLE SUMP DEPTH AVAILABLE

ANTI-FLOTATION DESIGN AVAILABLE UPON REQUEST



# FRAME AND COVER (DIAMETER VARIES) N.T.S.

<u>SITE SPECIFIC</u> DATA REQUIREMENTS							
					<u> </u>		
STRUCTURE ID							
WATER QUALITY	FLOW RAT	E (CF	S OR L/s)		*		
PEAK FLOW RAT	E (CFS OR	L/s)			*		
RETURN PERIOD	OF PEAK F	LOW	(YRS)		*		
SCREEN APERTU	JRE (2400 C	R 470	00)		*		
PIPE DATA:	I.E.	MA	TERIAL	D	IAMETER		
INLET PIPE 1	*		*		*		
INLET PIPE 2	*		*		*		
OUTLET PIPE	*		*		*		
RIM ELEVATION					*		
ANTI-FLOTATION	BALLAST		WIDTH		HEIGHT		
			*		*		
NOTES/SPECIAL REQUIREMENTS:							
* PER ENGINEER OF RECORD							

#### GENERAL NOTES

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- 2. DIMENSIONS MARKED WITH () ARE REFERENCE DIMENSIONS. ACTUAL DIMENSIONS MAY VARY.
- 3. FOR FABRICATION DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHTS, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS LLC REPRESENTATIVE. www.contechES.com
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- 6. PVC HYDRAULIC SHEAR PLATE IS PLACED ON SHELF AT BOTTOM OF SCREEN CYLINDER. REMOVE AND REPLACE AS NECESSARY DURING MAINTENANCE CLEANING.

#### **INSTALLATION NOTES**

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- B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE CDS MANHOLE STRUCTURE (LIFTING CLUTCHES PROVIDED).
- C. CONTRACTOR TO ADD JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS, AND ASSEMBLE STRUCTURE.
- D. CONTRACTOR TO PROVIDE, INSTALL, AND GROUT PIPES. MATCH PIPE INVERTS WITH ELEVATIONS SHOWN.
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CDS PMSU2015-4-C INLINE CDS STANDARD DETAIL

#### Appendix G – Water Demand



#### Domestic & Fire Protection Water Supply/Storage

Project: Lot 31 Clark Street
Town of the Blue Mountains

 Prepared by:
 C. Capes

 Checked by:
 C. Capes

 Project No:
 2021-185

 Date:
 December 13, 2023

#### **Domestic Flow Calculations**

#### Commercial & Industrial Building

Number of Water Fixture Units = <260

Water Demand = 2360 L/day OBC Table 7.4.10.5 Conversion of WFSU to L/day (minimum value 2360 L when WFU <260)

Operating Hours = 10 hrs

Average Day Demand = 2,360 L/day per building

0.59 L/s

Max Day Demand = 4,720 L/day Town Recommended Max Day Demand Factor = 2.0

Peak Hourly Demand = 1,062 L/hr Town Recommended Max Hour Demand Factor = 4.5

Domestic Peak Demand = 0.30 L/s per building

#### **Fire Flow Calculations**

#### Based on Fire Underwriters Survey

Total Domestic Peak Demand =

 $F = 220C\sqrt{A}$ 

Where F = Required fire flow in Lpm

C = Construction type coefficient = 1.5 Type V wood frame (essentially all combustible)

0.8 Type IV-A Mass Timber Construction
0.9 Type IV-B Mass Timber Construction

1.0 Type IV-C Mass Timber Construction1.5 Type IV-D Mass Timber Construction

1.0 ordinary construction (brick or other masonry walls, combustible floor and interior)

0.8 non-combustible (unprotected metal structure components, masonry or metal walls)

fire-resistive construction (fully protected frame, floors, roof)

#### A = Total floor area in sq.m. excluding basements, includes garage per building

Floor	Area (sq.m)	%
Bldg. A	1,301	100%
Bldg. A	1,301	100%
Bldg. B	454	100%
Bldg. C	681	100%
Bldg. D	764	100%
Bldg. E	1,054	100%
Bldg. F	764	100%
Bldg. G	530	100%
Bldg. H	1,704	100%
Total	8,553	

for fire resistive bldgs., consider the 2 largest adjoining floors + 50% of each of any floors immediately above them when the vertical openings are not adequately protected.

for fire registive

for fire resistive bldgs., consider the area of largest adjoining floor + 25% of each of the 2 floors immediately adjoining floors when the vertical openings and exterior vertical communications are protected for 1 hr rating.

Total Applicable Area = 8,553

F = 16,277 L/min (adjust formula accordingly)

16000 L/min (Round to nearest 1000 L/min)

2 Occupancy Reduction

-25% reduction for non-combustible

-15% reduction for limited combustible 0% reduction for combustible

15% increase for free burning 25% increase for rapid burning

Reduction = 0 L/min (0% of F1)

F = 16000 L/min

3 Sprinkler Reduction

30% Reduction for NFPA Sprinkler System (refer to FUS manual, 2020)

Reduction = 0 L/min (0% of F2)

F = 16000 L/min

Separation Charge

ocparation of	iaigo								
		Building A	Building B	Building C	Building D	Buildng E	Building F	Building G	<b>Building H</b>
	North Side	15%	0%	0%	0%	0%	0%	0%	0%
	East Side	0%	20%	20%	20%	20%	20%	0%	0%
	South Side	0%	15%	15%	15%	15%	15%	15%	0%
	West Side	0%	0%	20%	20%	20%	20%	20%	0%
•	Total	15%	35%	55%	55%	55%	55%	35%	0%

MaxSeparation Charge = 55% 8800 L/min (55% of F3)

Fire Flow = 1 - 2 - 3 + 4

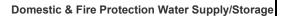
= 24800 L/min = 413.33 L/s

Min Value Under FUS = 2,000 L/min

Domestic Demand + Fire Flow = 413.92 L/s 24835 I/min Max Value Under FUS = 45,000 L/min

0 to 3m 25%
3.1 to 10m 20%
10.1 to 20m 15%
20.1 to 30m 0%

Separation Charge





Project: Lot 31 Clark Street **Town of the Blue Mountains**  Prepared by: C. Capes C. Capes Checked by: Project No: 2021-185 Date: December 13, 2023

#### **Domestic Flow Calculations**

#### Industrial Buildings

OBC Table 7.4.10.5 Conversion of WFSU to L/day (minimum value 2360 L when WFU <260) Number of Water Fixture Units = <260

2360 L/day Water Demand = Operating Hours = 10 hrs

Average Day Demand = per building 2,360 L/day

4,720 L/day Town Recommended Max Day Demand Factor = 2.0 Max Day Demand = Peak Hourly Demand = 1,062 L/hr Town Recommended Max Hour Demand Factor = 4.5

Domestic Peak Demand = 0.30 L/s per building Total Domestic Peak Demand = 0.59 L/s

#### **Fire Flow Calculations**

Office of the Fire Marshal, OFM Guideline, Fire Protection Water Supply Guideline for Part 3 in the Ontarion Building Code (Oct 1999) Subsection 3.2.2 of the Ontario Building Code, 2012

Q=KVS<sub>Total</sub> where

Q = Minimum supply of water in Litres (L)

K = water supply coefficient from Table 1

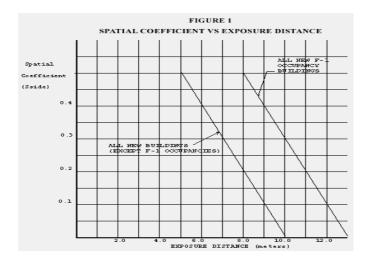
V = total building volume in cubic meters

 $S_{Tot}$  = total of the spacial coefficient values from the property line exposures on all sides as obtained from the formula:

 $S_{Tot} = 1.0 + [(S_{Side1}) + (S_{Side2}) + (S_{Side3}) + ... etc.]$ 

values are obtained from Figure 1, as modified by Sections 6.39(e) and 6.3(f) of the OBC Guideline where  $S_{\text{Side}}$ 

need not exceed 2.0  $S_{\text{Tot}}$ 



#### Building Classification:

Building is of noncombustible construction or of heavy timber construction conforming to Article 3.1.4.6. of the OBC. Floor assemblies are fire separations but with no fire-resistance rating. Roof assemblies, mezzanines, loadbearing walls, columns and arches do not have a fire-resistance rating.

Water Supply Coefficient - K

Table 1 of OBC A.3.2.5.7

Type F3, OBC Table 3.1.2.1

#### **Building Volumes**

Bldg.	Area	Height	Volume
	(m <sup>2</sup> )	(m)	(m <sup>3</sup> )
Bldg. A	1,301	7.50	9755
Bldg. B	454	3.00	1362
Bldg. C	681	3.00	2044
Bldg. D	764	3.00	2291
Bldg. E	1,054	3.00	3161
Bldg. F	764	3.00	2291
Bldg. G	530	3.00	1591
Bldg. H	1,704	4.00	6816
		Total	29310

Total Building Volume

Exposure Distances

$$S_{Tot} = 1.0 + [(S_{Side1}) + (S_{Side2}) + (S_{Side3}) + ... etc.]$$

	S <sub>Tot</sub>	S <sub>Side</sub> (W)	West	S <sub>Side</sub> (S)	South	S <sub>Side</sub> (E)	East	S <sub>Side</sub> (N)	North	Bldg.
			(m)		(m)		(m)		(m)	
	0	0	>10 m	Bldg. A						
	0	0	>10 m	Bldg. B						
	0.1	0	>10 m	0	>10 m	0.1	9 m	0	>10 m	Bldg. C
	0.5	0.1	9 m	0	>10 m	0.4	6 m	0	>10 m	Bldg. D
← Max S	0.8	0.4	6 m	0	>10 m	0.4	6 m	0	>10 m	Bldg. E
	0.5	0.4	6.00	0	>10 m	0.1	9.00	0	>10 m	Bldg. F
	0.1	0.1	9.00	0	>10 m	0	>10 m	0	>10 m	Bldg. G
1	0	0	>10 m	Bldg. H						
1										

S<sub>Tot</sub> 1.80 Max. Value = 2.0

**Minimum Fire Water Supply** 

Fire Water Supply Flow Rate

Q=KVS<sub>Total</sub> 1160671 Litres

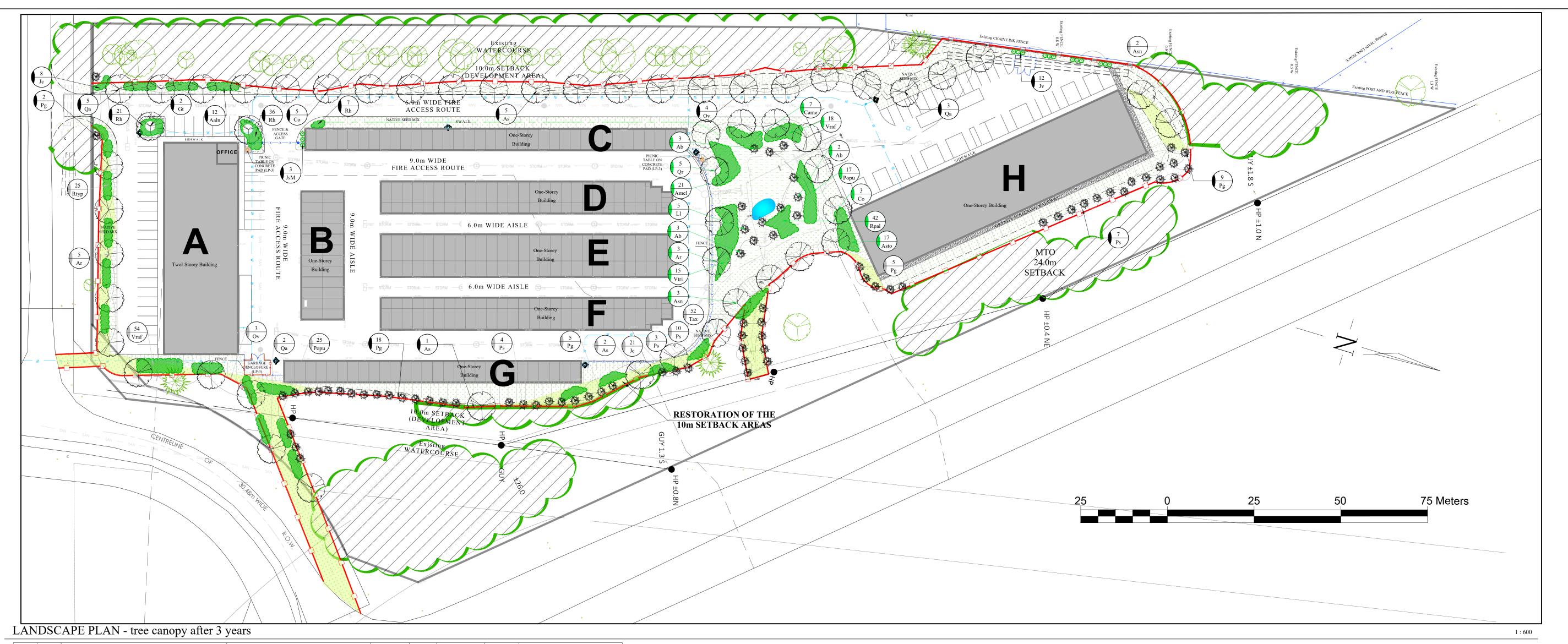
Table 2 Required Minimum Water Supply Flow Rate (L/min), provided in the OBC A.3.2.5.7

**150.00** L/s

Domestic + Fire Flow Rate

150.59 L/s

Appendix H – Landscape Plan



RESTORATION PLANT LABEL

PRESERVED TREE CANOPY

PICNIC TABLE ON CONCRETE PAD (LP-3)

RESTORATION AREA

GENERAL LEGEND

50 cm 3 gal 1.5m o/c D-3 Full form / Container grown

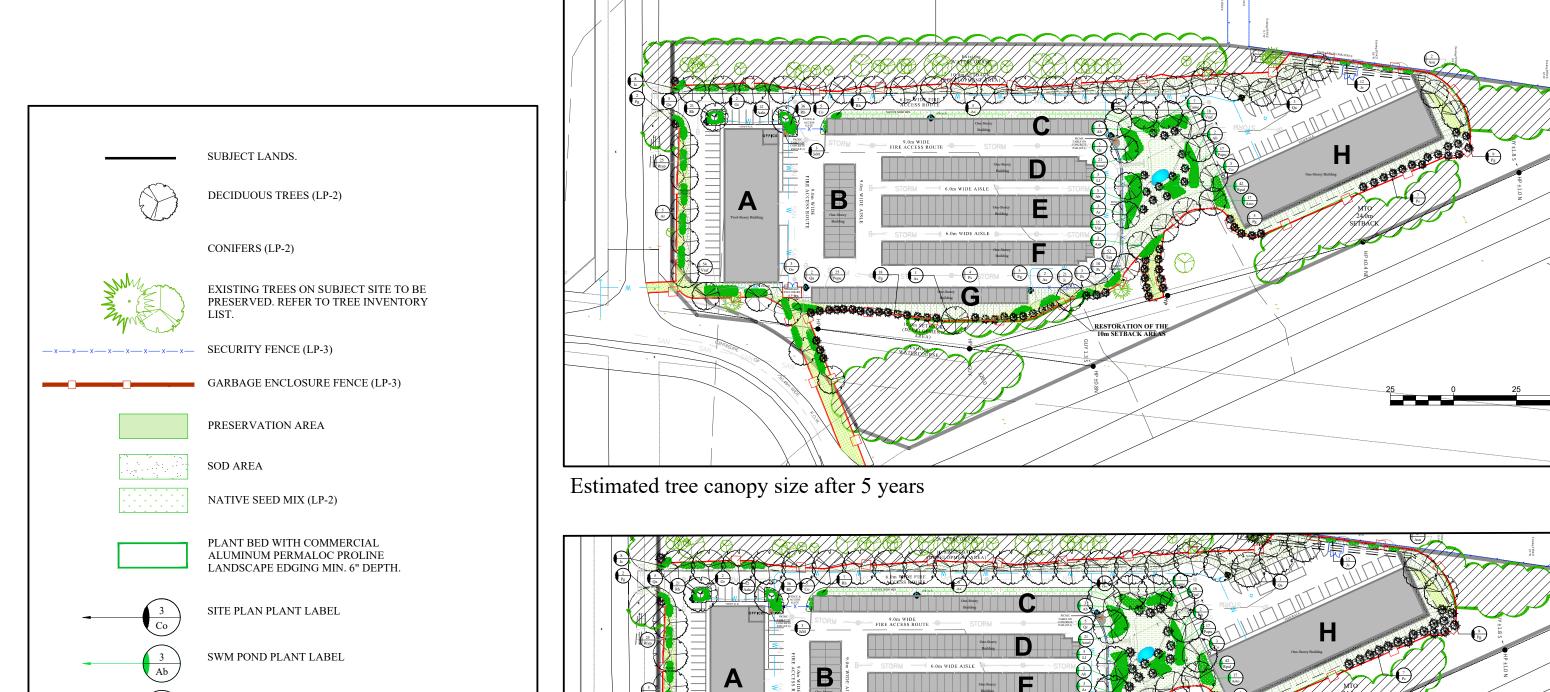
CODE QNTY COMMON NAME BOTANICAL NAME FORM SPACING DETAIL NOTES **DECIDUOUS TREES** D-1 Full Form/ Fall prunning only As 6 SUGAR MAPLE Acer saccharum W.B. 9.0 m o/c Co 5 COMMON HACKBERRY Celtis occidentalis W.B. 9.0 m o/c D-1 Full Form 60 mm Gt 2 SHADEMASTER HONEYLOCUST Gleditsia triacanthos var. inermis 'Shademaster' | 60 mm W.B. 9.0 m o/c D-1 Full Form/ Spring plant only Ov 4 IRONWOOD 60 mm W.B. 7.0 m o/c Ostrya virginiana Qa 8 WHITE OAK 60 mm W.B. 15.0 m o/c D-1 Spring planting/ Soil pH<7.5 Quercus alba CONIFEROUS TREES Pg 29 WHITE SPRUCE 200cm w.b. 5m o/c D-2 Full form / Do not cut leader Picea glauca Jv | 12 | EASTERN RED CEDAR D-2 Full form / Do not cut leader w.b. 2.5m o/c Juniperus virginiana Ps 7 EASTERN WHITE PINE Pinus strobus 150 cm w.b. 6m o/c D-2 Full form / Do not cut leader SHRUBS Aaln 12 SERVICE-BERRY Amelanchier alnifolia 60 cm 3 gal 1.8m o/c D-3 Full form / Container grown Jc 8 CANADIAN JUNIPER D-2 Full form / Container grown 40cm 3 gal 1.5m o/c Juniperus communis var. depressa JsM 3 MOFFET JUNIPER 150cm w.b. 1.5 m o/c D-3 Full form / Container grown Juniperus scopulorum 'Moffettii' NATIVE PERENNIALS Rh 64 BLACK EYED SUSAN Rudbeckia hirta 1 gal pot 0.90 m o/c n/a Full form / Container grown SITE PLAN PLANT LIST. REFER TO DETAILS AND NOTES ON LP-2 DECIDUOUS TREES D-1 Full Form/ Fall prunning only Ar 3 RED MAPLE 60 mm W.B. 9.0 m o/c Acer rubrum D-1 Full Form/ Fall prunning only Asn 3 SILVER MAPLE W.B. 9.0 m o/c Acer saccharinum Co 3 COMMON HACKBERRY 60 mm W.B. 9.0 m o/c D-1 Full Form Celtis occidentalis Qr 5 RED OAK 60 mm W.B. 9.0 m o/c D-1 Spring planting/ Soil pH<7.5 Quercus rubra CONIFEROUS TREES Ab 8 BALSAM FIR Abies balsamea 200cm w.b. 6m o/c D-2 Full form / Do not cut leader Ll 5 AMERICAN LARCH D-2 Full form / Do not cut leader Larix laricina 200cm w.b. 8.0m o/c NATIVE SHRUBS Amel 21 BLACK CHOKEBERRY D-3 Full form / Container grown 60 cm 3 gal 1.2m o/c Aronia melanocarpa Asto | 17 | RUNNING JUNEBERRY D-3 Full form / Container grown Amelanchier stolonifera 60 cm 3 gal 1.8m o/c Came 7 AMERICAN HAZELNUT Corylus americana 60 cm 3 gal 2.5m o/c D-3 Full form / Container grown Popu 17 COMMON NINEBARK Physocarpus opulifolius 60 cm 3 gal 1.8m o/c D-3 Full form / Container grown Rpal 42 SWAMP ROSE 60 cm 3 gal 1.0m o/c D-3 Full form / Container grown Rosa palustris Vraf 18 DOWNY ARROW-WOOD 60 cm 3 gal 1.5m o/c D-3 Full form / Container grown Viburnum rafinesquianum 60 cm 3 gal 2.5m o/c D-3 Full form / Container grown Vtri | 15 | HIGH BUSH CRANBERRY Viburnum trilobum SWM POND PLANT LIST. REFER TO DETAILS AND NOTES ON LP-2 DECIDUOUS TREES Ar 5 RED MAPLE D-1 Full Form/ Fall prunning only Acer rubrum W.B. 9.0 m o/c D-1 Full Form/ Fall prunning only As 2 SUGAR MAPLE Acer saccharum W.B. 9.0 m o/c Asn 2 SILVER MAPLE 60 mm W.B. 9.0 m o/c D-1 Full Form/ Fall prunning only Acer saccharinum D-1 Full Form/ Spring plant only Ov 3 IRONWOOD 45 mm W.B. 7.0 m o/c Ostrya virginiana Qa 2 WHITE OAK Quercus alba 45 mm W.B. 15.0 m o/c D-1 Spring planting/ Soil pH<7.5 CONIFEROUS TREES Pg 10 WHITE SPRUCE 150 cm w.b. 5m o/c D-2 Full form / Do not cut leader Picea glauca Ps 17 EASTERN WHITE PINE 150 cm w.b. 6m o/c D-2 Full form / Do not cut leader Pinus strobus NATIVE SHRUBS Jc 21 CANADIAN JUNIPER D-2 Full form / Container grown Juniperus communis var. depressa 40cm 3 gal 1.5m o/c Popu 25 COMMON NINEBARK 50cm 3 gal 1.8m o/c D-3 Full form / Container grown Physocarpus opulifolius 80cm3 gal2.5m o/cD-3Full form / Container grown40cm2 gal1.0 m o/cD-3Full form / Container grown Rtyp 25 STAGHORN SUMAC Rhus typhina Tax 52 AMERICAN YEW

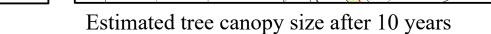
Taxus canadensis

Viburnum rafinesquianum

Vraf | 54 | DOWNY ARROW-WOOD

RESTORATION PLANT LIST. REFER TO DETAILS AND NOTES ON LP-2







#### GENERAL NOTES

CONTRACTOR IS RESPONSIBLE FOR ALL LOCATES INCLUDING ALL UNDERGROUND SERVICES PRIOR TO ANY EXCAVATION OR INSTALLATIONS. THE CONTRACTOR IS REQUIRED TO NOTIFY THE VARIOUS UTILITY COMPANIES 48 HOURS PRIOR TO THE COMMENCEMENT OF ANY WORK.

ANY ACCOMPANYING DOCUMENTATION RELATING TO THE LANDSCAPE PLAN AND/OR PRESERVATION PLAN SUCH AS TENDER DOCUMENTS AND CHANGE NOTICES ARE TO BE ENDORSED BY J.D.B. ASSOCIATES LIMITED PRIOR TO THE BEGINNING OF ANY SITE WORKS. IN THE EVENT THAT OF A DISCREPANCY THE DRAWING SHALL BE ASSUMED CORRECT.

IT IS THE RESPONSIBILITY OF THE PERSON OR PERSONS RESPONSIBLE FOR THE CONSTRUCTED WORKS TO NOTIFY THE LANDSCAPE ARCHITECT WHEN PREPARED FOR ANY REOUIRED INSPECTIONS AND SIGN OFFS.

SCHEDULED MEETINGS SHALL TAKE PLACE AT THE CLOSEST MUTUALLY CONVENIENT TIME.

No.	REVISION	DATE	APRVD
1.	CLIENT REVIEW	December 6 <sup>th</sup> , 2022	StT
2.	AS PER TOWN COMMENTS	December 27 <sup>th</sup> , 2023	StT
3.	AS PER TOWN COMMENTS	May 28 <sup>th</sup> , 2024	StT
4.	AS PER TOWN COMMENTS	August 1st, 2024	StT
5.	AS PER UPDATED SITE PLAN	November 14 <sup>th</sup> , 2024	StT

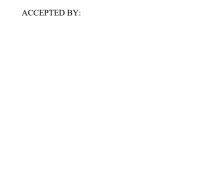
ALL DRAWINGS AND SPECIFICATIONS ARE INSTRUMENTS OF SERVICE AND ARE THE PROPERTY OF JDB ASSOCIATES LIMITED. DRAWINGS ARE NOT TO BE MODIFIED AND/OR REPRODUCED WITHOUT THE WRITTEN CONSENT OF JDB ASSOCIATES LIMITED REPRODUCTION OF DRAWINGS IN ANY FORM WITHOUT THE CONSENT OF JDB ASSOCIATES LIMITED VOIDS THE DRAWING AT WHICH TIME JDB ASSOCIATES LIMITED ACCEPTS NO LIABILITY FOR THE DRAWING CONTENT OR WORKS RESULTING FROM SAID REPRODUCTION. DRAWINGS MAY BE REPRODUCED BY MUNICIPAL AND GOVERNMENT AGENCIES RESPONSIBLE FOR APPROVALS FOR THEIR OWN USE. JDB ASSOCIATES RESERVES THE RIGHT TO WITHDRAW ANY DRAWING(S) FROM GOVERNMENT OR MUNICIPAL AGENCIES WHETHER APPROVED OR NOT IN THE EVENT THAT ACCOUNTS ARE NOT SETTLED OR REMAIN OUTSTANDING

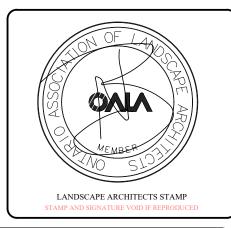
IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY ALL DIMENSIONS ON THE SITE AND REPORT ANY DISCREPANCIES OR VARIATIONS FROM THE JDB ASSOCIATES LIMITED IS NOT RESPONSIBLE FOR THE ACCURACY OF SURVEY, ARCHITECTURAL, MECHANICAL, ENGINEERING OR ELECTRICAL INFORMATION SHOWN ON THE DRAWING. FOR FURTHER INFORMATION REFER TO APPROPRIATE SURVEY, ARCHITECTURAL, MECHANICAL, ENGINEERING OR

ELECTRICAL DRAWINGS PRIOR TO PROCEEDING WITH ANY WORKS.

THIS DRAWING IS NOT TO BE SCALED.



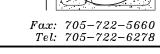




JDB associates LTD.

Urban Designers Landscape Architects

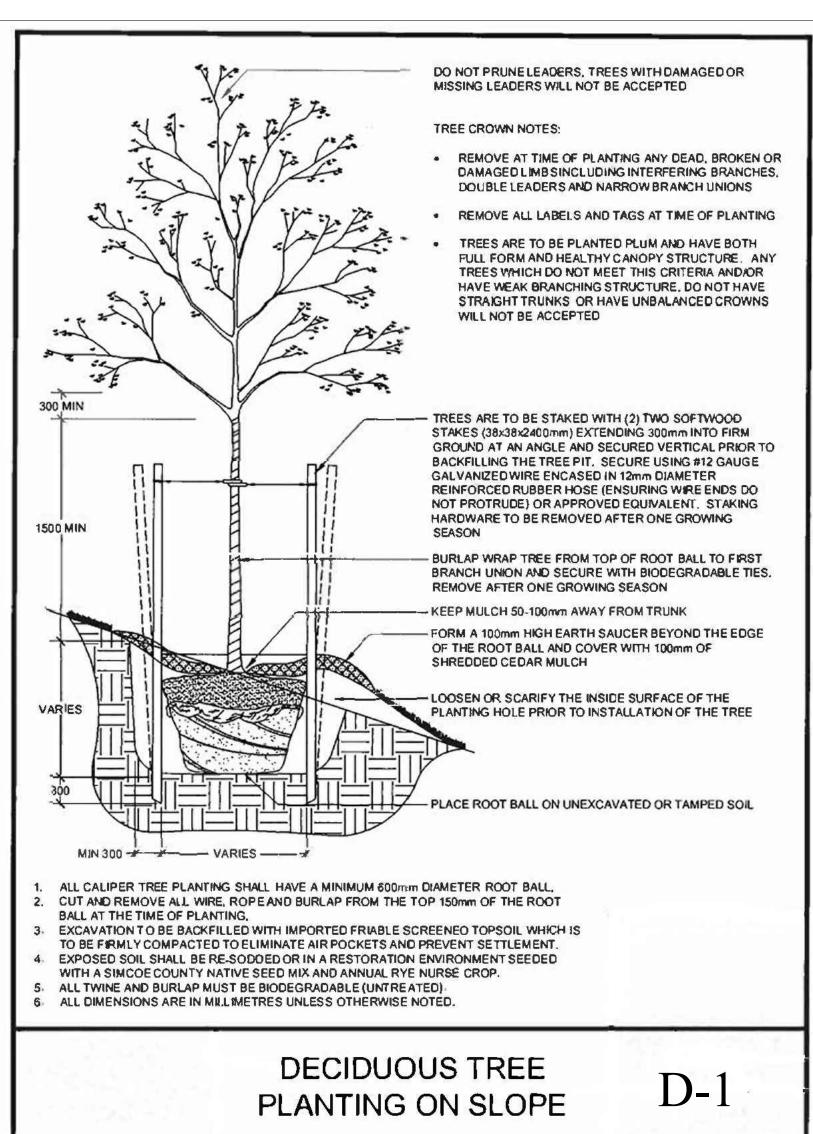
274 Burton Ave., Suite 1201 Barrie, Ontario L4N 5W4

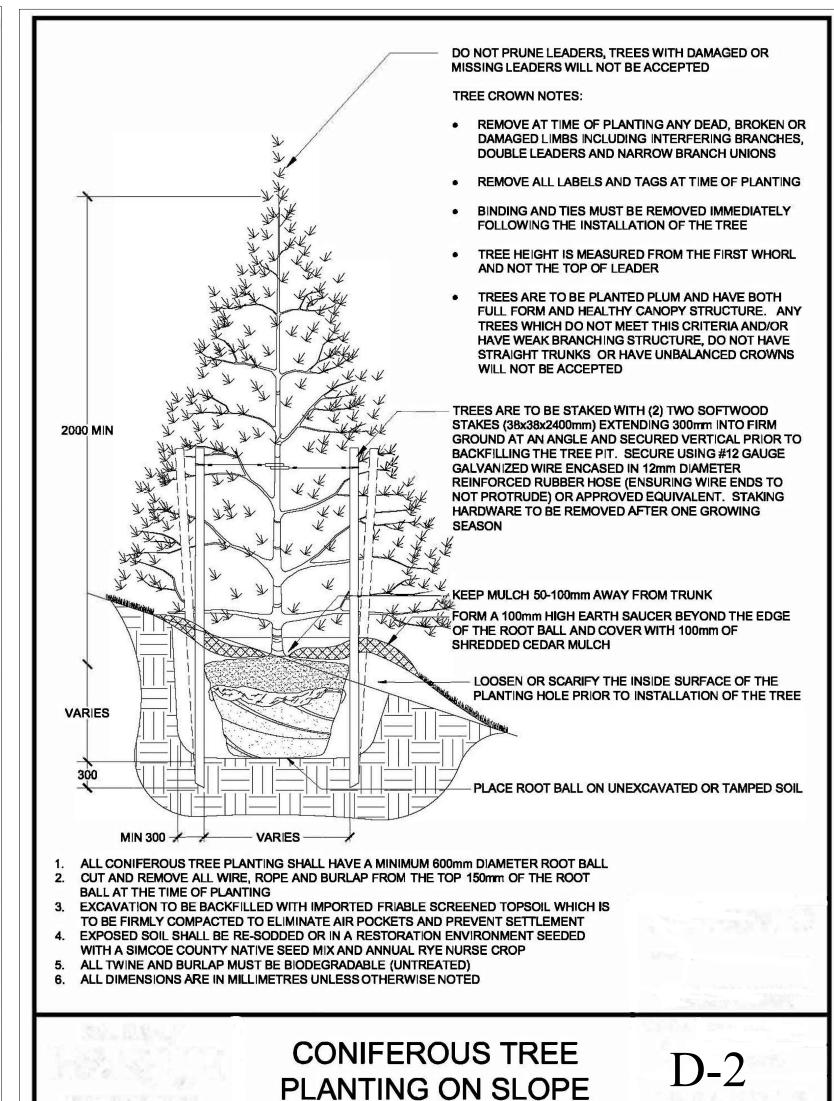


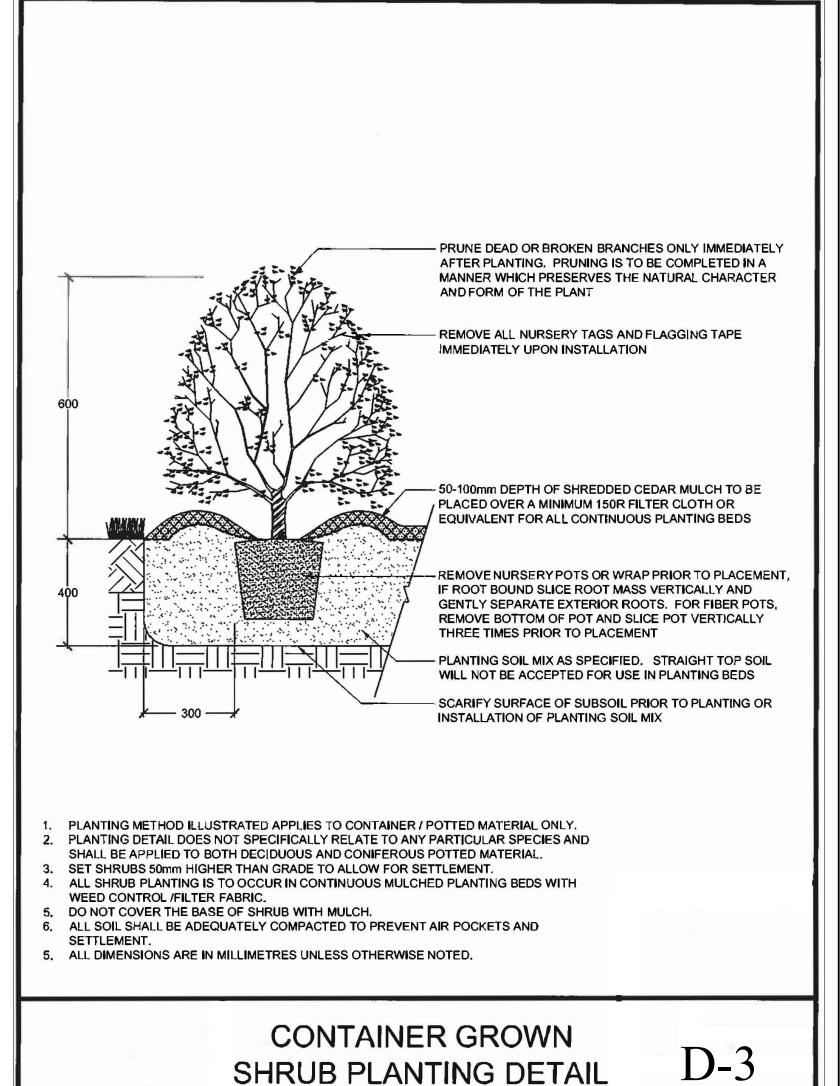
LOT 31 CLARK ST Town of the Blue Mountains, ON

LANDSCAPE/ RESTORATION PLANTING PLAN

SCALE:	DESIGNED BY:	REVIEWED BY:
As shown	IT	NB
TOWN FILE No.	OUR FILE REF. # 14-22	LP-1







SHORELINE ZONE NO-MOW ZONE

APPROXIMATELY 837 sq/m (+10%) (+/- MEASUREMENTS TO BE TAKEN

#7107 - SEASONALLY FLOODED ANNUAL/PERENNIAL MIXTURE

- 5% NODDING BUR MARIGOLD (Biddens ceruna 22% FOX SEDGE (Carex vulpinoidea
- 25% CANADA WILD RYE (Elymus canadensis)
- 25% SWITCHGRASS (Panicum virgatum) 23% FOWL MEADOWGRASS (Poa palustris)

SEEDING RATE - 30kg PER HECTARE SUPPLIED BY - OSC SEEDS 1-519-886-055

ANNUAL RYE NURSE CROP TO BE APPLIED AT TIME OF SEASONALL FLOODED ANNUAL/PERENNIAL MIXTURE AT A RATE OF 12kg PER HECTARE. REFER TO TERRASEEDING NOTES AND MULCH APPLICATION SPECIFICATIONS IN THIS BOX FOR FURTHER DETAILS

NB - ALL DISTURBED AREAS TO BE TERRASEEDED

### UPLAND PLANTING ZONE

NO-MOW ZONE (INCLUDING ACCESS ROAD) APPROXIMATELY 5,500 sq/m (+10%) (+/- MEASUREMENTS TO BE

SIMCOE COUNTY NATIVE UPLAND MIXTURE

- 2% NEW ENGLAND ASTER (Aster novae-anglaie) 12% BLACK EYED SUSAN (Rudbeckia hirta)
- 20% SAND DROPSEED (Sporobolus crytandrus 20% CANADA WILD RYE (Elymus candadensis)
- 4% CANADA GOLDEN ROD (Solidago canadensis 5% COMMON MILKWEED (Asclepias syriaca)
- 1% WILD BERGAMONTE (Monarda Fistulosa
- 1% SMOOTH BLUE ASTER (Aster laevis) 15% LITTLE BLUE STEM (Andropogon scoparius)

20% INDIANGRASS (Sorghastrum nutans)

SUPPLIED BY - OSC SEEDS 1-519-886-0557 ANNUAL RYE NURSE CROP TO BE APPLIED AT TIME OF NATIVE UPLAND PLANTING MIXTURE AT A RATE OF 12kg PER HECTARE. REFER TO TERRASEEDING NOTES AND MULCH APPLICATION SPECIFICATIONS IN THIS BOX FOR FURTHER DETAILS.

#### TERRASEEDING APPLICATION SPECIFICATIONS

ECOBLANKET OR TERRASEEDING TACTIFIER / FLEXGUARD HYDROMULCH SHALL BE APPLIED AS PER MANUFACTURER SPECIFICATIONS. SEE NOTES ON THIS PAGE.

MINIMUM DEPTH OF 0.45m FOR TERRESTRIAL AREAS EXCEPT THE ACCESS ROAD BASE (5cm) AND MINIMUM 0.35m FOR TEMPORARY UNDERWATER AREAS. TOPSOIL OUALITY SHALL BE AS PER OPSS 570, IT SHALL BE TESTED BY AN INDEPENDENT LABORATORY PRIOR TO

SEEDING AND TOP SOIL NOTES

IN THE SWM POND AREA TOPSOIL SHALL BE PROVIDED TO A

CLIENT REVIEW December 6<sup>th</sup>, 2022 StT AS PER TOWN COMMENTS December 27<sup>th</sup>, 2023 StT AS PER TOWN COMMENTS May 28th, 2024 AS PER TOWN COMMENTS August 1st, 2024 AS PER UPDATED SITE PLAN November 14th, 2024 StT

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CHANGE NOTICES ARE TO BE ENDORSED BY LDB ASSOCIATES LIMITED

PRIOR TO THE BEGINNING OF ANY SITE WORKS. IN THE EVENT THAT OF A

IT IS THE RESPONSIBILITY OF THE PERSON OR PERSONS RESPONSIBLE FOR

THE CONSTRUCTED WORKS TO NOTIFY THE LANDSCAPE ARCHITECT WHEN

SCHEDULED MEETINGS SHALL TAKE PLACE AT THE CLOSEST MUTUALLY

DATE APRV

COMPANIES 48 HOURS PRIOR TO THE COMMENCEMENT OF ANY WORK.

DISCREPANCY THE DRAWING SHALL BE ASSUMED CORRECT.

PREPARED FOR ANY REQUIRED INSPECTIONS AND SIGN OFFS.

THE CONTRACTOR IS REQUIRED TO NOTIFY THE VARIOUS UTILITY

ALL DRAWINGS AND SPECIFICATIONS ARE INSTRUMENTS OF SERVICE AND ARE THE PROPERTY OF JDB ASSOCIATES LIMITED. DRAWINGS ARE NOT TO BE MODIFIED AND/OR REPRODUCED WITHOUT THE WRITTEN CONSENT OF JDB ASSOCIATES LIMITED. REPRODUCTION OF DRAWINGS IN ANY FORM WITHOUT THE CONSENT OF JDB ASSOCIATES LIMITED VOIDS THE DRAWING AT WHICH TIME JDB ASSOCIATES LIMITED ACCEPTS NO LIABILITY FOR THE DRAWING CONTENT OR WORKS RESULTING FROM SAID REPRODUCTION DRAWINGS MAY BE REPRODUCED BY MUNICIPAL AND GOVERNMEN' AGENCIES RESPONSIBLE FOR APPROVALS FOR THEIR OWN USE. JDB ASSOCIATES RESERVES THE RIGHT TO WITHDRAW ANY DRAWING(S) FROM GOVERNMENT OR MUNICIPAL AGENCIES WHETHER APPROVED OR NOT IN THE EVENT THAT ACCOUNTS ARE NOT SETTLED OR REMAIN OUTSTANDING

IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY ALL DIMENSIONS ON THE SITE AND REPORT ANY DISCREPANCIES OR VARIATIONS FROM THE SUPPLIED INFORMATION TO THE LANDSCAPE ARCHITECT WITH THE PROJECT. JDB ASSOCIATES LIMITED IS NOT RESPONSIBLE FOR THE ACCURACY OF SURVEY, ARCHITECTURAL, MECHANICAL, ENGINEERING OR ELECTRICAL INFORMATION SHOWN ON THE DRAWING. FOR FURTHER INFORMATION REFER TO APPROPRIATE SURVEY, ARCHITECTURAL, MECHANICAL, ENGINEERING OR ELECTRICAL DRAWINGS PRIOR TO PROCEEDING WITH ANY WORKS.

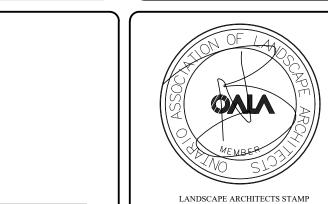
THIS DRAWING IS NOT TO BE SCALED.

**GENERAL NOTES** 

BASE PLAN REVISED: MARCH. 2022 Client info:

IPS- INNOVATIVE PLANNING SOLUTIONS 647 Welham Road, Unit 9A, Barrie, ON,

ACCEPTED BY



L4N 0B7

JDB associates LTD. Urban Designers

Arborists274 Burton Ave., Suite 1201 Barrie, Ontario



LOT 31 CLARK ST

Town of the Blue Mountains, ON

LANDSCAPE PLAN DETAILS

L4N 5W4

REVIEWED BY: NTS TOWN FILE No. OUR FILE REF. # LP-2

BLOWN ON COMPOSTED ORGANICS INJECTED WITH MICRO BLEND WITH SEED MIX MIN. 25mm (1") APPLICATION ON SLOPE UP TO 3:1 (H:V) MIN. 50mm (2") APPLICATION ON SLOPES 3:1 TO 2:1 (H:V) POTTED PLANTS TO BE INSTALLED PRIOR TO ECO-BLANKET APPLICATION BARE ROOT PLANTS INSTALLED PRIOR TO ECO-BLANKET APPLICATION -NO FOOT TRAFFIC ON ECO-BLANKET FOR ONE FULL GROWING SEASON AFTER

storm water conveyance systems

This work shall consist of furnishing, constructing and maintaining an EcoBlanket( to Rexius specifications. EcoBlanket is a ground cover (surface blanket) of the Rexius specified compost/mulch (Erosion Blend) combined with a special additive (Microblend) constructed with a pneumatic blower to control and reduce soil erosion. An EcoBlanket stabilizes the soil, prevents splash, sheet and rill erosion, and removes suspended soil particles and contaminants from water moving off the site and into adjacent waterways or

1.1. This EcoBlanket must be applied by Landsource Organix Ltd., 100 Britannia Road East, Hornby, Ontario L0P 1E0, tollfree 1 877 548 8558 (toll free fax 1 877 548 8559) or equivalent certified EcoBlanket installer 1.2. Materials must be applied using a pneumatic blower unit complete with a supplemental granular injection system capable of installing at least 15 cubic meters per hour.

1.3. Contractor must have at least 3 years of proven experience in successfully installing EcoBlanketsTM.

The EcoBlanket filtering material consists of the Rexius Erosion Blend of compost and mulch materials, according to the Rexius particle sizing specifications, in combination with the Rexius Microblend additive.

2.1. Particle size must meet exact specifications of the Rexius EcoBlanket Erosion Blend material supplied by a certified supplier/installer 2.2. The compost portion of EcoBlanket shall be derived from well-decomposed organic matter source produced by controlled aerobic (biological) decomposition that has been sanitized through the generation of heat and stabilized to the point that it is appropriate for this particular application. Compost material shall be processed through proper thermophilic composting, meeting the Canadian Council of Ministers of the Environment's (CCME) definition for a 'process to further reduce pathogens' (PFRP). The compost portion shall meet the chemical, physical and biological properties (as outlined in the chart on reverse). These and all other required properties for the performance of the EcoBlanket are included in the Rexius EcoBlanket Manufacture Guidelines followed by certified suppliers/installers. 2.3. Rexius Microblend additive shall be injected into Erosion Blend material at time of EcoBlanket construction.

2.4. A proof of certification as an EcoBlanket supplier shall be submitted to the Landscape Architect for approval prior to installation. Test results for EcoBlanket performance shall be made available upon request. 2.5. Where seeding or planting is planned, Erosion Blend material must meet Rexius' minimum specification requirements for seeding purposes.

1. The EcoBlanket shall be placed as shown on the plans or as directed by the Landscape Architect.

3.2. On areas with slopes 3:1 to 2:1 (H:V) the EcoBlanket shall be uniformly applied directly at the soil surface with a pneumatic blower as specified by Rexius. EcoBlanket shall be applied at a depth of 50 mm minimum and approximately 90 cm over the top of the slope, or overlap it into existing vegetation. On areas with slopes up to 3:1(H:V) the EcoBlanket shall be applied at a depth of 25mm minimum. In extreme conditions and where specified by the Engineer/Landscape Architect., EcoBerms shall be added and constructed at the top of the slope in parallel intervals down the profile of the slope (6 metres to 9 metres apart) if necessary. (The Engineer/Landscape Architect shall specify

3.3. Rexius Microblend shall be applied/injected at a minimum rate of 615 kgs. per hectare (or as specified by Rexius), to be confirmed by inspector/project manager. 3.4. EcoBlanket application depth may be modified based on specific site (e.g., soil characteristics, existing vegetation) and climatic conditions, as well as particular project related requirements. The severity of slope grade, as well as slope length will also influence the addition of EcoBerms and number of EcoBerm placements in combination with 3.5. If temporary or long-term vegetation is required, Erosion Blend material may be injected with seed during application. The Engineer/Landscape Architect shall specify seed

requirements and the compost/mulch component shall abide by the minimum standards set by Rexius for seeding 3.6. Where vegetation is to be established, slightly roughen (scarify) slopes and remove large clods, rocks, stumps, roots larger than 50 mm in diameter and debris on slopes. This soil preparation step may be eliminated where approved by the Landscape Architect/Designer, or where seeding or planting is not planned. Where practical, track (compact) perpendicular to contours on the slope using a bulldozer before applying EcoBlanket injected with seed. 3.7. Do not use EcoBlankets in areas of concentrated flow (ie. ditches, streams, etc.)

3.8. Unless otherwise allowed by Landscape Architect, seeding shall be performed within the local region's seeding deadlines.

The Contractor shall maintain the EcoBlanket in a functional condition at all times. Contractor shall make periodic inspections of the EcoBlanket for effectiveness and shall immediately correct all deficiencies. Where deficiencies exist, additional EcoBlanket material shall be installed immediately to required depth.

5.0 Method of Measurement:

EcoBlanket shall be measured by the square metre, complete in place.

6.1. Place EcoBlankets on denuded areas immediately or as directed by Landscape Architect. EcoBerms and/or temporary or permanent vegetation shall be applied/established when necessary, along with other appropriate structural measures and controls, for additional erosion and sediment control. 6.2. The work specified in this Section consists of designing, providing, and maintaining erosion and sedimentation controls as necessary. All existing and foreseeable future conditions that affect the work inside and outside the site limits must be acknowledged as the Contractor's responsibility.

6.3. Contractor is responsible for providing effective sediment control measures based on performance. Contractor may, with approval from the Landscape Architect, work outside the minimum construction requirements to establish a working erosion control system.

Parameters 1,4 Reported as (units of measure) EcoBlanket to be Vegetated EcoBlanket to be left Un-vegetated PH2 pH units 5.0 - 8.5 N/A Soluble Salt Concentration2 (electrical conductivity) ds/m (mmhos/cm) Maximum 5 N/A

1. Recommended test methodologies are provided in Test Methods for the Examination of Composting and Compost (SCC through BNQ) 2. Each specific plant species requires a specific pH range. Each plant also has a salinity tolerance rating, and maximum tolerable quantities are known. When specifying the establishment of any plant or turf species, it is important to understand their pH and soluble salt requirements, and how they relate to the compost in use. 3. Stability/Maturity rating is an area of compost science that is still evolving, and as such, other various test methodscould be considered. Also, never base compost quality conclusions on the result of a single stability/maturity test.

- INSPECTION AND WARRANTY 1. GIVE TIMELY NOTICE TO THE LANDSCAPE ARCHITECT FOR THE REQUIRED START UP SITE INSPECTION TO REVIEW SITE CONDITIONS
- 3. THE LANDSCAPE ARCHITECT RESERVES THE RIGHT TO REJECT ANY PLANTS, WHETHER INSTALLED OR NOT, WHICH DO NOT CONFORM TO THE SPECIFICATIONS AND/OR SITE DRAWING. REMOVE ALL REJECTED PLANTS FROM THE SITE IMMEDIATELY. DO NOT REMOVE ANY LABELS FROM PLANTS UNTIL PLANTS HAVE BEEN INSPECTED AND APPROVED
- 5. FINAL ACCEPTANCE OF THE PROJECT WILL BE CARRIED OUT UPON COMPLETION OF ALL WORK INCLUDED IN THE CONTRACT.

## TOPSOIL REQUIREMENTS (LANDSCAPING AREAS)

- 18 m<sup>3</sup> FOR LARGE TREES - 12 m<sup>3</sup> FOR SMALL TREES
- SIX PARTS OF FERTILE LOAM SOIL (50-60% SAND, 20-40% SILT, 6-10% CLAY, 2-5% ORGANIC), WITH A pH OF 7.5\* OR LESS, FREE OF CLAY LUMPS, DEBRIS, TOXIC SUBSTANCES, STONES, WOODY MATERIAL, WEED SEEDS AND GRASS ROOTS
- ONE PART OF WELL-ROTTED FARM MANURE
- 8. PREPARED SOIL MIXTURE FOR SOD AREAS SHALL CONSIST OF MIN. 20cm HIGH QUALITY SOIL: SIX PARTS OF FERTILE LOAM SOIL (50-60% SAND, 20-40% SILT, 6-10% CLAY, 2-5% ORGANIC), WITH A pH OF 7.5\* OR LESS, FREE OF CLAY LUMPS, DEBRIS, TOXIC SUBSTANCES, STONES, WOODY MATERIAL,

ALL TREES ARE TO BE STAKED OR GUY WIRED ACCORDING TO DETAILS PROVIDED. NO ACCESSIBLE OPEN HOLE TREE PITS SHALL BE PERMITTED OVERNIGHT. WEED SEEDS AND GRASS ROOTS . REMOVE BURLAP AND ROPE FROM THE TOP 1/3 OF ROOT BALLS. . WATER AT TIME OF PLANTING AND WHENEVER DEEMED NECESSARY TO MAINTAIN THE TREES IN A

ALL PLANT MATERIALS WHICH CAN NOT BE PLANTED IMMEDIATELY UPON ARRIVAL ON SITE SHALL BE PROPERLY HEELED IN OR WELL PROTECTED WITH SOIL OR SIMILAR MATERIALS TO PREVENT DRYING OUT AND SHALL BE KEPT MOIST UNTIL COMMENCEMENT OF PLANTING

VERIFY ALL EXISTING SITE CONDITIONS AND REPORT ANY DISCREPANCIES BEFORE COMMENCING WORK.

TO SERVICES, EXISTING VEGETATION OR ANY OTHER FEATURES TO BE RETAINED.

SOD AREAS TO RECEIVE 200MM DEPTH OF TOPSOIL UNLESS OTHERWISE INDICATED.

AND REPLACED AS PER TOPSOIL REOUIREMENTS.

PLANT MATERIAL REQUIREMENTS

UNLESS OTHERWISE STATED

IN THE PLANT SCHEDULE

ASSUMED TO BE CORRECT.

THE CONTRACTOR SHALL VERIFY THE LOCATION OF ALL UTILITIES AND IS RESPONSIBLE FOR ANY DAMAGE

FOR ALL AREAS OF DISTURBANCE: NATIVE TOPSOIL IS TO BE STRIPPED, STOCK PILED, LABORATORY TESTED

UNIFORM SPECIMENS. NO SUBSTITUTIONS WILL BE PERMITTED WITHOUT WRITTEN APPROVAL FROM THE

DECIDUOUS TREES SHALL HAVE A STRAIGHT CENTRAL LEADER AND WELL BRANCHED 1.5M ABOVE GRADE

CONIFEROUS TREES SHALL HAVE A STRAIGHT CENTRAL LEADER AND DENSELY BRANCHED TO WITHIN 0.3m

PLANT MATERIAL TO BE NO.1 GRADE NURSERY STOCK. UNSATISFACTORY STOCK WILL BE REFUSED ON THE

SOD TO BE CANADA NO.1 NURSERY SOD, MEETING ONTARIO SOD GROWERS ASSOCIATION STANDARDS. ALL

. ALL PLANT MATERIAL WHICH ARE SPECIFIED BY O.C. (ON CENTER SPACING) ARE TO BE PLANTED AS NOTED

ALL TREE PITS SHALL INCLUDE TREATMENT WITH MICORRHIZAL FUNGLOF THE WALLS BEFORE PLANTING

LOCATIONS FOR PLANT MATERIAL AND PLANTING BEDS ARE TO BE MARKED OR STAKED OUT BY THE

CONTRACTOR AND APPROVED BY THE LANDSCAPE ARCHITECT AND MUNICIPAL STAFF PRIOR TO

. IN THE EVENT OF A DISCREPANCY BETWEEN THE PLANT LIST AND DRAWING, THE DRAWING WILL BE

ALL MASS PLANTINGS OF SHRUBS SHALL BE IN CONTINUOUS BEDS AND MULCHED AS SPECIFIED.

(2L OF "MIKE" OR SIMILAR PRODUCT SHALL BE USED FOR EACH 60MM CALIPER TREE)

ALL PLANTING MATERIAL AND OPERATIONS TO MEET OR EXCEED THE HORTICULTURAL STANDARDS OF THE CANADIAN NURSERY LANDSCAPE ASSOCIATION AND THE HORTICULTURAL TRADES ASSOCIATION. ALL

PLANT MATERIAL LISTED IN THE PLANT SCHEDULE ARE MINIMUM SIZES +/- NURSERY GROWN AND

NO ACCESSIBLE OPEN HOLE TREE PITS SHALL BE PERMITTED OVERNIGHT. ALL OPEN PITS SHALL BE ADEQUATELY PROTECTED BY BARRIERS OR FILLED IN WITH SOIL PRIOR TO THE END OF EACH PLANTING

9. ALL NEW WORK TO BLEND NEATLY AND SMOOTHLY WITH EXISTING CONDITIONS.

SITE CONDITIONS

PLANTING NOTES

- AND SCHEDULE FOLLOW UP INSTALLATIONS
- 2. INSTALLATION OF PLANT MATERIAL PRIOR TO INSPECTION BY THE LANDSCAPE ARCHITECT WILL BE THE CONTRACTOR'S RESPONSIBILITY.
- BY THE LANDSCAPE ARCHITECT
- 4. ALL PLANT MATERIAL AND WORKMANSHIP WILL BE INSPECTED AND IS UNDER WARRANTY FOR A MINIMUM OF TWO YEAR FROM DATE OF WRITTEN ACCEPTANCE. ALL PLANT MATERIAL MUST BE IN A HEALTHY, VIGOROUS GROWING CONDITION SATISFACTORY TO THE CONTRACT ADMINISTRATOR AT THE END OF THE WARRANTY PERIOD
- OR BE REPLACED AT THE CONTRACTORS EXPENSE.

- 6. PROVIDE MINIMUM SOIL VOLUME PER TREE AT 0.45m DEPTH FOR STREET TREES (OR ABOVE UTILITIES) AND MAX. 0.9m IN OTHER
- 7. PREPARED SOIL MIXTURE FOR EACH TREE PITS AND SHRUBS SHALL CONSIST OF HIGH OUALITY SOIL
- ONE PART COARSE PULVERIZED CANADIAN PEAT MOSS

PLANTING NOTES (REFER TO "SEEDING AND TOPSOIL NOTES" ON THIS PAGE FOR THE SWM POND AREA)

- BASE OF PLANTS TO BE SITUATED FLUSH WITH FINISH GRADE OF ECO-BLANKET

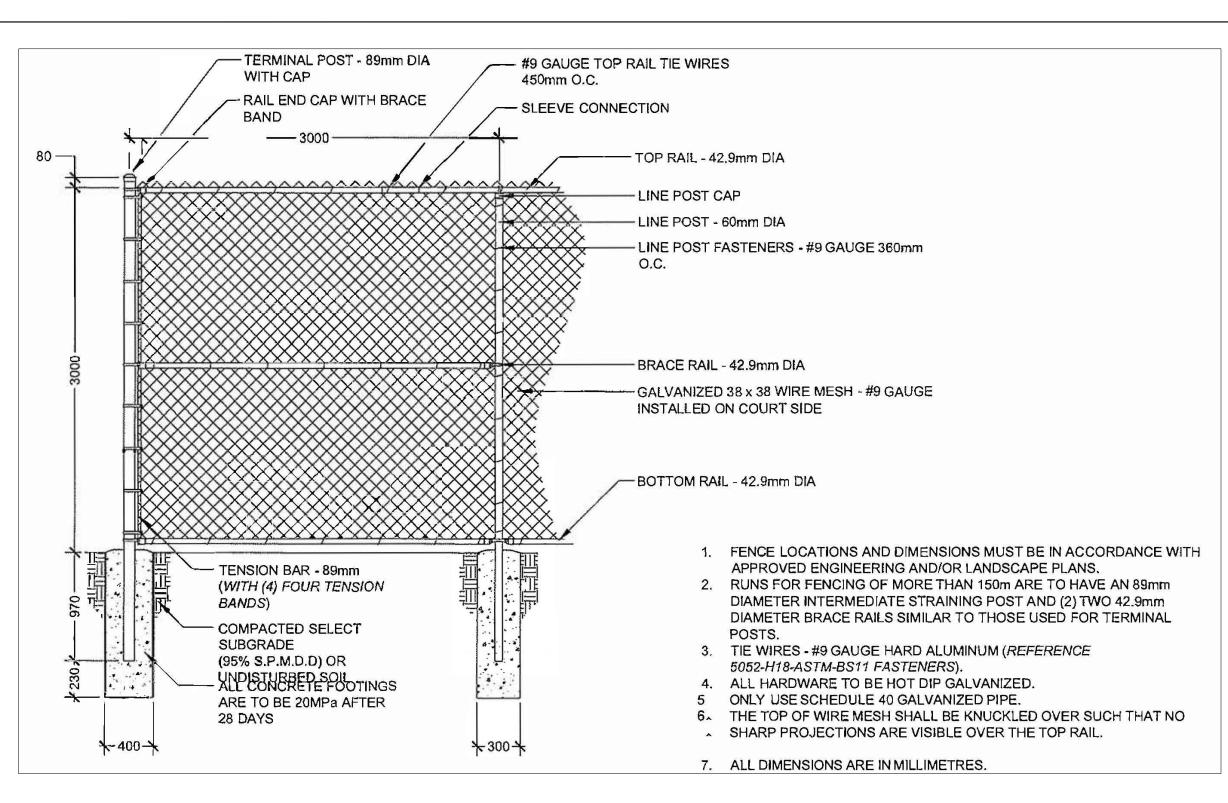
PREPARED SURFACE

Stability3 Carbon Dioxide Evolution Rate mg CO2-C per g OM per day < 8 N/A Physical Contaminants (man-made inerts) % dry weight basis < 1 < 1

DETAIL ADAPTED FROM LANDSOURCE ORGANIX EcoBlanket (tel.:1-877-548-8558)

4. Landscape Architect may modify the allowable compost specification ranges based on specific field conditions and plant requirements.

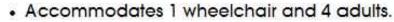
ECOBLANKET SPECIFICATIONS AND DETAIL



SECURITY CHAIN LINK FENCE DETAIL - black vinyl coated

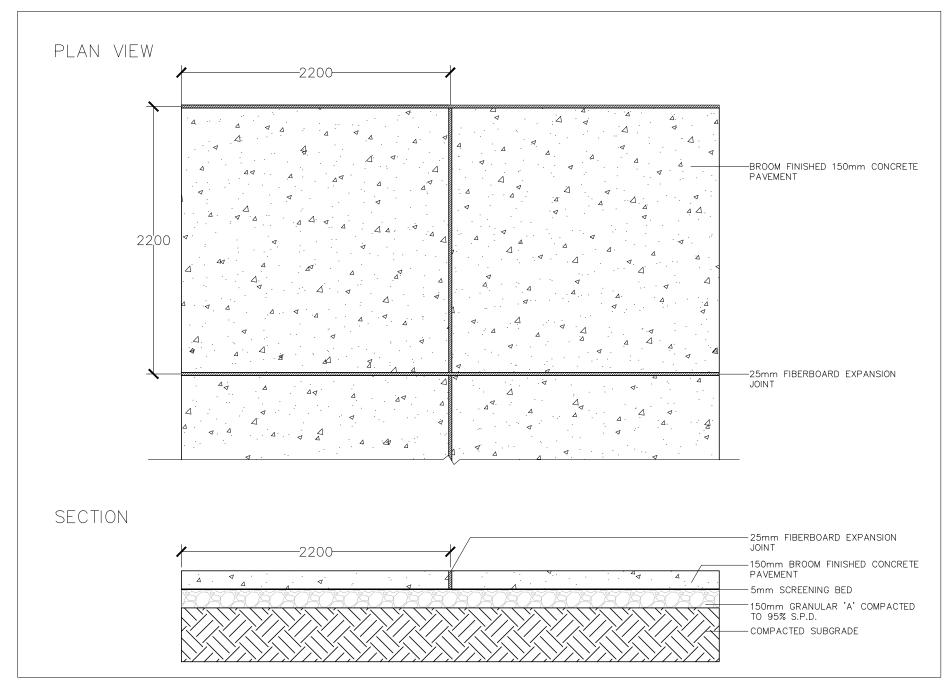
# ADA Hex Recycled Plastic Picnic Table - 46", Cedar

Virtually maintenance free! Recommended for warehouse patios, campgrounds, schools and parks.

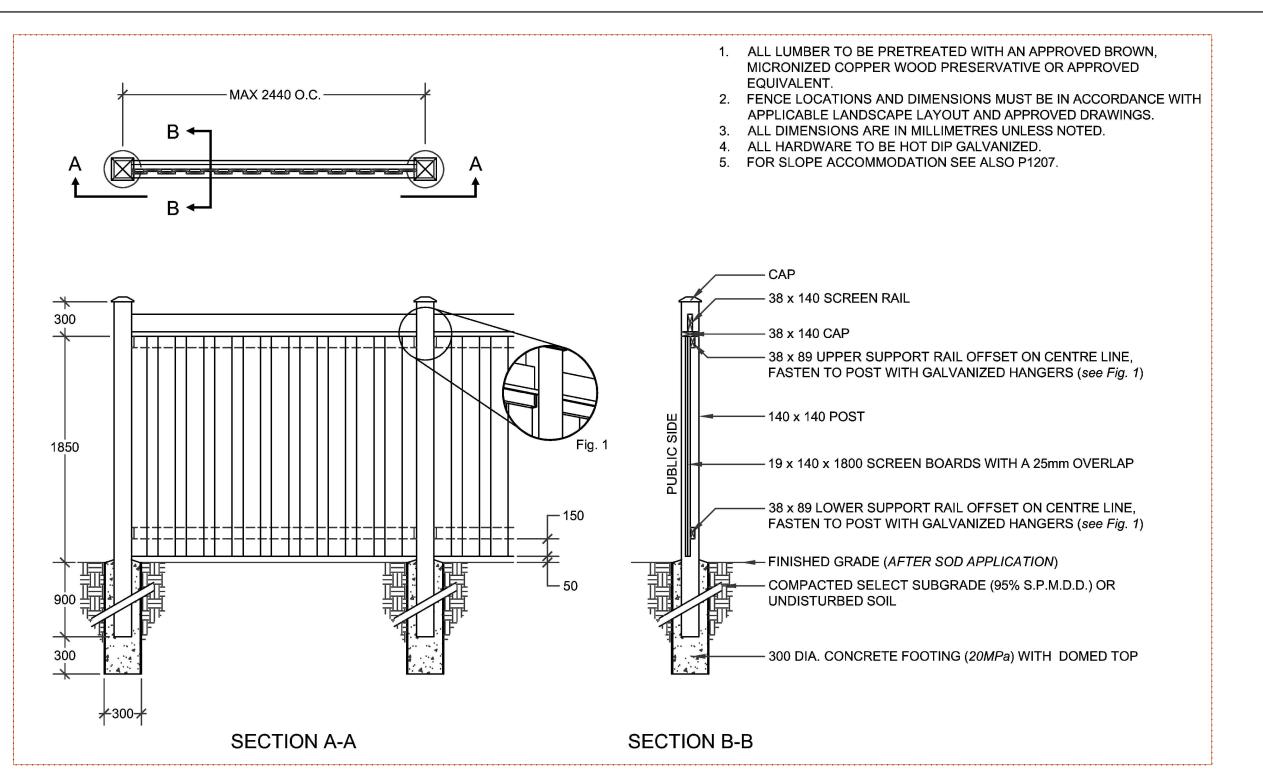


- 100% recycled UV-protected plastic won't rot, splinter or crack. Withstands harsh weather.
- Natural-looking wood material never needs sanding, sealing, painting or staining.
- Durable 2" thick planks predrilled for easy assembly.
- H-6681 Mounting Hardware available.

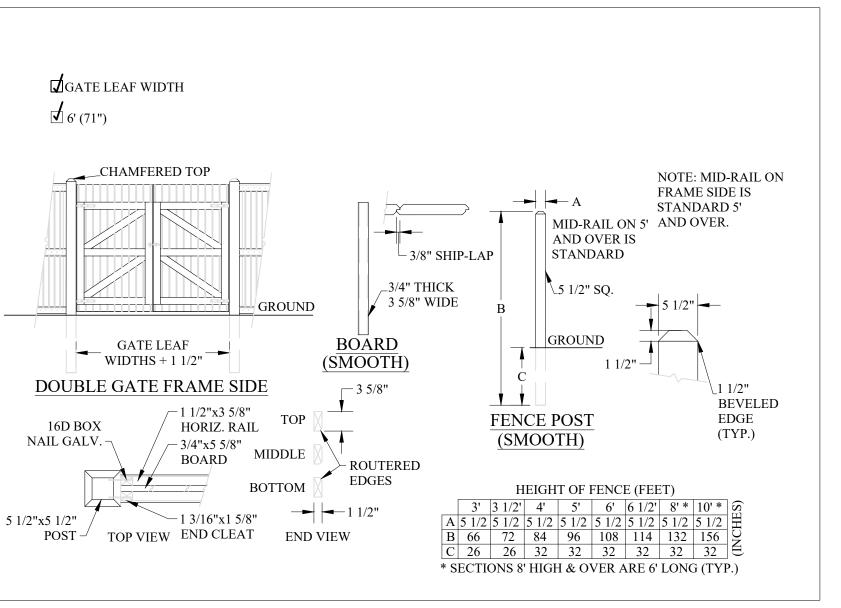
PICNIC BENCH U-LINE MODEL H-6575 ON CONCRETE PAD (BELOW)



CONCRETE PAD DETAIL (typ.)



GARBAGE ENCLOSURE FENCE DETAIL



GATE DETAIL (typ.) - GATE STYLE TO MATCH THE FENCE STYLE AND SPECS.

SHOP DRAWING TO BE PROVIDED TO THE LANDSCAPE ARCHITECT FOR APPROVAL BEFORE CONSTRUCTION



#### GENERAL NOTES

CONTRACTOR IS RESPONSIBLE FOR ALL LOCATES INCLUDING ALL UNDERGROUND SERVICES PRIOR TO ANY EXCAVATION OR INSTALLATIONS. THE CONTRACTOR IS REQUIRED TO NOTIFY THE VARIOUS UTILITY COMPANIES 48 HOURS PRIOR TO THE COMMENCEMENT OF ANY WORK.

ANY ACCOMPANYING DOCUMENTATION RELATING TO THE LANDSCAPE PLAN AND/OR PRESERVATION PLAN SUCH AS TENDER DOCUMENTS AND CHANGE NOTICES ARE TO BE ENDORSED BY J.D.B. ASSOCIATES LIMITED PRIOR TO THE BEGINNING OF ANY SITE WORKS. IN THE EVENT THAT OF A DISCREPANCY THE DRAWING SHALL BE ASSUMED CORRECT.

IT IS THE RESPONSIBILITY OF THE PERSON OR PERSONS RESPONSIBLE FOR THE CONSTRUCTED WORKS TO NOTIFY THE LANDSCAPE ARCHITECT WHEN PREPARED FOR ANY REQUIRED INSPECTIONS AND SIGN OFFS.

SCHEDULED MEETINGS SHALL TAKE PLACE AT THE CLOSEST MUTUALLY CONVENIENT TIME.

No.	REVISION	DATE	APRVD
1.	CLIENT REVIEW	December 6 <sup>th</sup> , 2022	StT
2.	AS PER TOWN COMMENTS	December 27 <sup>th</sup> , 2023	StT
3.	AS PER TOWN COMMENTS	May 28 <sup>th</sup> , 2024	StT
4.	AS PER TOWN COMMENTS	August 1st, 2024	StT
5.	AS PER UPDATED SITE PLAN	November 14th, 2024	StT

ALL DRAWINGS AND SPECIFICATIONS ARE INSTRUMENTS OF SERVICE AND ARE THE PROPERTY OF JDB ASSOCIATES LIMITED. DRAWINGS ARE NOT TO BE MODIFIED AND/OR REPRODUCED WITHOUT THE WRITTEN CONSENT OF JDB ASSOCIATES LIMITED. REPRODUCTION OF DRAWINGS IN ANY FORM WITHOUT THE CONSENT OF JDB ASSOCIATES LIMITED VOIDS THE DRAWING AT WHICH TIME JDB ASSOCIATES LIMITED ACCEPTS NO LIABILITY FOR THE DRAWING CONTENT OR WORKS RESULTING FROM SAID REPRODUCTION. DRAWINGS MAY BE REPRODUCED BY MUNICIPAL AND GOVERNMENT AGENCIES RESPONSIBLE FOR APPROVALS FOR THEIR OWN USE. JDB ASSOCIATES RESERVES THE RIGHT TO WITHDRAW ANY DRAWING(S) FROM GOVERNMENT OR MUNICIPAL AGENCIES WHETHER APPROVED OR NOT IN THE EVENT THAT ACCOUNTS ARE NOT SETTLED OR REMAIN OUTSTANDING.

IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY ALL DIMENSIONS ON THE SITE AND REPORT ANY DISCREPANCIES OR VARIATIONS FROM THE SUPPLIED INFORMATION TO THE LANDSCAPE ARCHITECT WITH THE PROJECT. JDB ASSOCIATES LIMITED IS NOT RESPONSIBLE FOR THE ACCURACY OF SURVEY, ARCHITECTURAL, MECHANICAL, ENGINEERING OR ELECTRICAL INFORMATION SHOWN ON THE DRAWING. FOR FURTHER INFORMATION REFER TO APPROPRIATE SURVEY, ARCHITECTURAL, MECHANICAL, ENGINEERING OR ELECTRICAL DRAWINGS PRIOR TO PROCEEDING WITH ANY WORKS.

THIS DRAWING IS NOT TO BE SCALED.



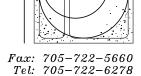
ACCEPTED BY:



JDB associates LTD.

Urban Designers Landscape Architects Arborists

274 Burton Ave., Suite 1201 Barrie, Ontario L4N 5W4



Tel: 705-722-62

LOT 31 CLARK ST

Town of the Blue Mountains, ON

LANDSCAPE/ RESTORATION PLANTING PLAN

SCALE:	DESIGNED BY:	REVIEWED BY:				
1:600	IT	NB				
TOWN FILE No.	OUR FILE REF. # 14-22	LP-3				

Appendix I – Sanitary Design

Project:	Lot 31 Clark Street
Municipality:	Town of the Blue Mountains
Project No.:	2021-185
Analyzed by:	KG
Date:	November 18, 2024
Manning n Value	0.013

#### Sanitary Sewer Design Sheet

#### Town of the Blue Mountains



	DESIGN SEWAGE FLOWS										SANITARY SEWER CAPACITY						
Location of Section	Cumulative Sanitary Catchment Area (ha)	From Upstream MH #	To Downstream MH #	Cumulative Serviced Population Cap.	Peaking Factor	Average Flow L/s	Peak Flow L/s	Peak Flow Infiltration: L/s	Total Peak Flow Infiltration L/s	Total Peak Flow L/s	Pipe Length m	Pipe Diameter mm	Pipe Grade %	Full Flow. Cap. L/s	Full Flow Velocity m/s	Percentage Full	
Bulding H	0.38	Building H	MH01A	n/a	4.50	0.21	0.95	0.087	0.087	1.04	15.0	150	2.00	21.5	1.22	4.82%	
Internal Access Road	0.98	MH01A	MH02A	n/a	4.50	0.54	2.45	0.225	0.225	2.68	15.1	200	1.00	32.8	1.04	8.16%	
Internal Access Road	1.06	MH02A	MH03A	n/a	4.50	0.59	2.65	0.244	0.244	2.89	82.7	200	0.50	23.2	0.74	12.48%	
Internal Access Road	2.06	MH03A	MH04A	n/a	4.50	1.14	5.15	0.474	0.474	5.62	81.7	200	0.50	23.2	0.74	24.25%	
Internal Access Road	2.23	MH04A	MH05A	n/a	4.50	1.24	5.58	0.513	0.513	6.09	70.4	200	0.50	23.2	0.74	26.25%	
Internal Access Road	2.23	MH05A	MH06A	n/a	4.50	1.24	5.58	0.513	0.513	6.09	26.2	200	3.00	56.8	1.81	10.72%	
Grey Road 2	2.23	MH06A	MH09A	n/a	4.50	1.24	5.58	0.513	0.513	6.09	27.2	200	0.50	23.2	0.74	26.25%	
Grey Road 2	0.25	MH07A	MH08A	n/a	4.50	0.14	0.63	0.058	0.058	0.68	44.9	200	0.50	23.2	0.74	2.94%	
Grey Road 2	0.25	MH08A	MH09A	n/a	4.50	0.14	0.63	0.058	0.058	0.68	32.8	200	0.50	23.2	0.74	2.94%	
Grey Road 2	2.73	MH09A	MH10A	n/a	4.50	1.52	6.83	0.628	0.628	7.45	36.6	200	0.50	23.2	0.74	32.14%	
Hwy. 26	2.73	MH10A	MH11A	n/a	4.50	1.52	6.83	0.628	0.628	7.45	89.5	200	0.50	23.2	0.74	32.14%	
Hwy. 26	2.73	MH11A	EX. SAN MH1	n/a	4.50	1.52	6.83	0.628	0.628	7.45	5.3	200	0.50	23.2	0.74	32.14%	
Cedar Run Wakeboard Cable Pk	5.00	Future Site	EX. SAN MH1	n/a	4.50	0.07	0.32	1.150	1.150	1.47	100.0	200	0.50	23.2	0.74	6.32%	
Hwy. 26	7.73	Ex. SAN MH1	Ex. SAN MH2	n/a	4.50	1.59	7.14	1.778	1.778	8.92	101.2	250	0.35	35.2	0.72	25.35%	
Hwy. 26	7.73	Ex. SAN MH2	Ex. SAN MH3	n/a	4.50	1.59	7.14	1.778	1.778	8.92	81.0	250	0.36	35.7	0.73	25.00%	
Hwy. 26	7.73	Ex. SAN MH3	Ex. SAN MH4	n/a	4.50	1.59	7.14	1.778	1.778	8.92	61.0	250	0.26	30.3	0.62	29.41%	
Hwy. 26	7.73	Ex. SAN MH4	Ex. SAN MH5	n/a	4.50	1.59	7.14	1.778	1.778	8.92	67.0	250	0.31	33.1	0.67	26.94%	
Hwy. 26	7.73	Ex. SAN MH5	Ex. SAN MH6	n/a	4.50	1.59	7.14	1.778	1.778	8.92	72.0	250	0.31	33.1	0.67	26.94%	
Hwy. 26	7.73	Ex. SAN MH6	Ex. SAN MH7	n/a	4.50	1.59	7.14	1.778	1.778	8.92	92.0	250	2.25	89.2	1.82	10.00%	
Hwy. 26	7.73	Ex. SAN MH7	Ex. SAN MH8	n/a	4.50	1.59	7.14	1.778	1.778	8.92	117.0	250	0.44	39.4	0.80	22.61%	
Lakeshore Road E.		Ex. SAN MH8	Ex. SAN MH	n/a		<u> </u>	**Measured Peal	Flow of 8.0L/s	  -  accumulated here	16.92	72.0	450	0.30	156.1	0.98	10.83%	
Lakeshore Road E.		Ex. SAN MH	Ex.SPS	n/a						24.92	8.0	450	0.30	156.1	0.98	15.96%	

NOTE:

AVERAGE DAILY PER CAPITA FLOW = 20,000 L/ha/day (light industrial land)

EXTRANEOUS FLOW ALLOWANCE = 0.23 L/sec/gross hectare

PEAKING FACTOR: Assumed 4.5 (max. recommended by the Town)

MANNING "n" = 0.013